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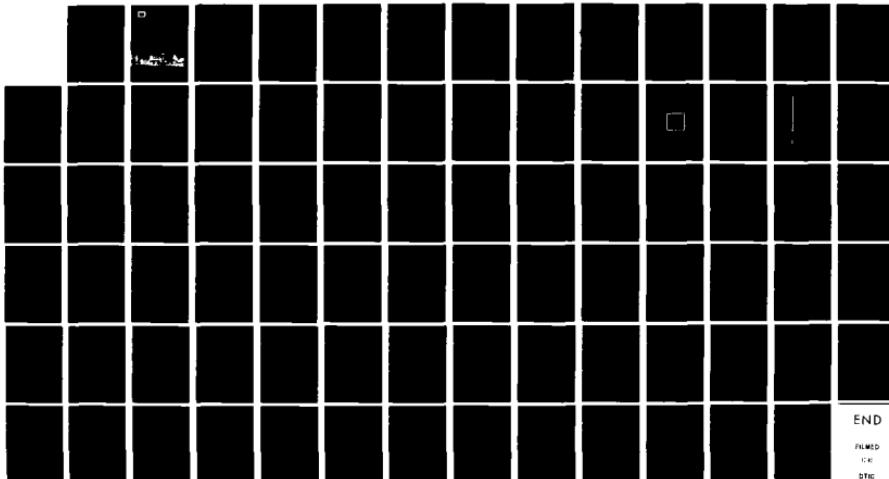
PRELIMINARY INVESTIGATION OF A PROPOSED PEARL RIVER
CUTOFF THROUGH THE OL. (U) ARMY ENGINEER WATERWAYS
EXPERIMENT STATION VICKSBURG MS GEOTE.

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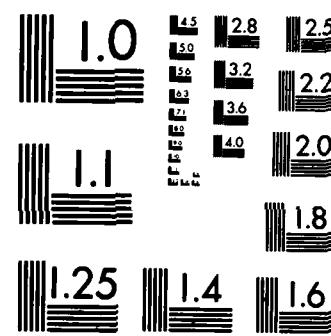
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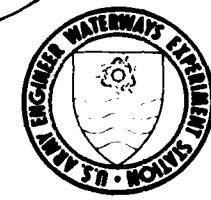
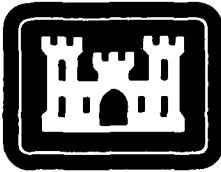
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PRELIMINARY INVESTIGATION OF A PROPOSED PEARL RIVER CUTOFF THROUGH THE OLD JACKSON SANITARY LANDFILL

by

Don R. Alexander, Charlie Whitten

Geotechnical Laboratory
U. S. Army Engineer Waterways Experiment Station
P. O. Box 631, Vicksburg, Miss. 39180

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| 20. ABSTRACT (Continue on reverse side if necessary and identify by block number) Solutions are presently being sought to reduce flooding along the Pearl River in the Jackson, Miss. area. Geotechnical considerations for one particular alternative are reported herein. A cutoff channel through the old Jackson Sanitary Landfill would considerably reduce the stage of the Pearl River. Problems stemming from environmental restrictions might include lining the channel with a low permeability material to prevent leachate infiltration from the landfill into the river. Soil borings were made and samples obtained at various depths along the center line to characterize the existing (Continued) ➤ | | | |

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soil conditions. Stability analyses were performed for the desired side slopes. Quantities of cut and required backfill were computed based on cross section and profile data obtained along the proposed center line.

Results from this report will be used by the U. S. Army Engineer District, Mobile in preparing a cost estimate to be used in a feasibility study for the proposed Pearl River cutoff channel. ←

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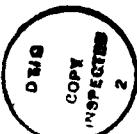
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Preface

The feasibility study described in this report was requested by Intra-Army Order for Reimbursable Services (DA 2544) No. FC-82-0017 from the U. S. Army Engineer District, Mobile, Alabama, dated 17 November 1981, to the Commander and Director, U. S. Army Engineer Waterways Experiment Station (WES), CE, Vicksburg, Mississippi.

The testing and analysis were conducted by personnel of the Geotechnical Laboratory (GL), WES, under the general supervision of Dr. W. F. Marcuson III, Chief of GL, Dr. P. F. Hadala, Assistant Chief of GL, and Mr. H. H. Ulery, Acting Chief of Pavement Systems Division (PSD); and under the direct supervision of Messrs. J. W. Hall, Jr., and A. J. Bush III, PSD. Slope stability analyses were performed with the aid of Mr. Yu Shih Jeng, Soil Mechanics Division. The geological portion of the study was written by Mr. Charlie Whitten, Engineering Geology and Rock Mechanics Division. The report was prepared by Mr. D. R. Alexander, PSD.

The Commander and Director of WES during the conduct of the investigation and preparation of this report was COL Tilford C. Creel, CE. The Technical Director was Mr. Fred R. Brown.



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Conversion Factors, U. S. Customary to Metric (SI)
Units of Measurement

U. S. customary units of measurement used in this report can be converted to metric (SI) units as follows:

| <u>Multiply</u> | <u>By</u> | <u>To Obtain</u> |
|------------------------------|-----------|---------------------------|
| acres | 4046.856 | square metres |
| cubic feet | 0.2831685 | cubic metres |
| cubic yards | 0.7645549 | cubic metres |
| feet | 0.3048 | metres |
| feet per second | 0.3048 | metres per second |
| inches | 25.4 | millimetres |
| miles (U. S. statute) | 1.609344 | kilometres |
| pounds (mass) | 0.4535924 | kilograms |
| pounds (mass) per cubic foot | 16.01846 | kilograms per cubic metre |
| square feet | 0.9290304 | square metres |

PRELIMINARY INVESTIGATION OF A PROPOSED
PEARL RIVER CUTOFF THROUGH THE OLD JACKSON
SANITARY LANDFILL

Background

1. The U. S. Army Engineer District, Mobile, Alabama, is currently evaluating several alternatives to reduce flooding near Jackson, Mississippi. One alternative involves excavating a channel through the old Jackson Sanitary Landfill. The proposed Pearl River cutoff is in southwest Jackson in the SW 1/4, S22, T5N, R1E (Figure 1). Figure 2 shows a location sketch of the landfill, which was closed in the mid-1970's. The site referred to in the text includes the landfill of approximately 127 acres.* The industrial waste generated in the Jackson area includes, but is not limited to, waste from printing; plastic manufacturing; household cleaning compound manufacturing; cottonseed oil production; insulation manufacturing; wood, metal, and cardboard containers manufacturing; paint manufacturing and operations; meat and other food processing; metal fabrication; furniture manufacturing; tool and die operations, bottling and canning industry; concrete and asphalt industry; wood preserving; machinery manufacturing; electrical generators; light bulbs and fixtures manufacturing; used oil refining; and others that are identifiable through the Mississippi Manufacturers Association's directory and city tax and permit records. Problems associated with excavating a channel through a potentially hazardous landfill area are assessed. The geotechnical information provided in this report will be used by the Mobile District in conjunction with a study performed by the Environmental Laboratory of the U. S. Army Engineer Waterways Experiment Station (WES) to evaluate the overall feasibility of the proposed cutoff. The final evaluation will be addressed in a survey report for the Pearl River Basin.

Purpose

2. The purpose of this study is to characterize the existing soil conditions

* A table of factors for converting U. S. customary units of measurement to metric (SI) units is presented on page 3.

along the proposed channel route, analyze data, and provide design recommendations. Special consideration has been given to the particular problems associated with excavation through a sanitary landfill. The channel design must be such that little or no seepage of potentially harmful leachate from the landfill into the river will occur. There apparently has been no consideration of the occurrence of general area flooding with consequent bank overtopping and landfill leachate entering the entire floodplain. This channel design is based on geotechnical considerations only. An estimate of the required volume of earth-work was made based on a preliminary channel design.

Field Investigations

Preliminary

3. Before the field investigation, boring data in close proximity to the site were obtained from the Mississippi Highway Department (MHD) and the Mississippi Geological Survey (MGS). The MHD foundation and channel change borings are 0.75 miles north of the site along the Interstate 20 and 55 bridges over the Pearl River floodplain (see Appendix A).

4. A chronological sequence of stereo aerial photographs (1940, 1949, 1955, 1959, 1965, 1972, and 1979) was obtained from the National Archives and the U. S. Department of Agriculture. Topographic maps published in 1963 and 1980 were obtained from the U. S. Geological Survey (USGS).

Site description

5. The site is located in the Pearl River floodplain and is bounded by the Pearl River to the northeast, east, and south. The prelandfill surface contained point bar deposits ranging from el 245.0 to 265.0.* Small stream channels from the uplands cut into the northwest and southwest corners of the site before emptying into the Pearl River.

6. The topography was totally changed by the addition of the landfill material. Aerial photographs show that approximately 20 percent of the site area along the western edge was being actively used as a landfill site in 1965. Data

* All elevations (el) cited herein are in feet referred to mean sea level.

from the 1963 USGS topographic maps show that approximately 1×10^5 cu yd of landfill material had been added to the site. Increased use of the site from 1965 until its closure in the mid-1970's changed the topography from a relatively flat floodplain to gently rolling hills ranging from el 250.0 to 295.0. A comparison of 1963 and 1980 USGS topography maps shows that approximately 2.1×10^6 cu yd of landfill material were placed on the site during that time period. A total of approximately 2.2×10^6 cu yd of landfill material has been added to the site.

7. The proposed channel route is crossed by power transmission lines approximately 600 ft from the north end and by an 18-in. gas line buried 54 in. approximately 600 ft from the south end.

Survey data

8. Rod and level data were obtained at 100-ft intervals along the center line of the proposed channel. The profile is plotted in Figure 3. Station 0+00 is located at the north end of the channel route.

Drilling and sampling

9. A two-phase drilling program was conducted to (a) install groundwater sampling wells and (b) obtain samples of the strata to a depth of 10 ft below the proposed channel invert of el 227.0. Two wells, W-1 and W-2, were drilled upstream (north of site) for background water quality, and four wells, W-3 through W-6, were drilled along the center line of the proposed channel cut (see Appendix B for boring logs). No soil samples were collected from the well borings. Seven splitspoon sampling holes (DH-1 through DH-7) were drilled along the proposed channel cut (see Appendix C). Representative samples were obtained from standard penetration splitspoon tests (SPT) at 5-ft intervals from the top of natural ground (prelandfill surface) to a minimum elevation of 217.0. Figure 4 presents the locations, offsets, and a summary of the boring data, and Figures 5-11 show the soil profiles from each boring.

10. Concrete blocks, steel beams, chain link fences, and other such items were encountered throughout the landfill material. Boring DH-6 had to be re-located seven times before the boring could be completed.

Stratigraphy

11. The local stratigraphy consists of Holocene or Recent Alluvium overlying Tertiary units. The MHD borings north of the site show 15 to 30 ft of

alluvial clay, silt, sand, and pea gravel overlying the Eocene Moodys Branch and Cockfield formations (Table 1). The Upper Eocene Yazoo clay is the Tertiary unit underlying the landfill site.* The MGS electric log file of boring N-10, the Filtrol Corporation Deep Monitor Well No. 1 located 0.75 miles west of the site, shows that the Yazoo clay is approximately 100 ft thick with its base at el 137.0.

12. The stratigraphic column along the channel cut consists of up to 30 ft of landfill material, overlying 20 to 25 ft of Recent Alluvium, which overlies the Upper Eocene Yazoo clay. The Recent Alluvium consists of a fining upward sequence of clay, silt, sand, and pea gravel. A typical vertical section of alluvium would have up to 10 ft of clay or silt overlying up to 25 ft of fine-to-medium sand with less than 1 ft of sandy pea gravel at the base. Fine-to-medium sand is the predominant material in the alluvium.

13. The Yazoo clay is a light green (light gray when dry), fossiliferous, stiff, plastic, montmorillonitic (highly expansive) clay. The top of the unit varies from el 233.0 to 238.0, except at boring DH-6 where the elevation dips to 224.0 in a shallow channel-like feature. The deepest penetration in the Yazoo clay was approximately 25 ft in borings DH-1, DH-2, and DH-3.

Groundwater

14. The water table at the site is primarily controlled by the rise and fall of the Pearl River. On 6 January 1982, the water table varied from el 250.0 to 255.0. Heavy rains the last week of January 1982 brought the Pearl River to bank-full stage. Corresponding rises in the water table were noted in the wells. Well W-3, located on the riverbank, was approximately 5 ft under water at that time. No perched water tables were encountered in the highly permeable landfill material.

Laboratory Tests

15. Jar samples obtained from the seven splitspoon sample holes were submitted to the laboratory for classification tests. Sieve analyses were performed

* W. H. Moore, 1965, "Hinds County Geology and Mineral Resources," Bulletin 105, Mississippi Geological Economic and Topographical Survey, Jackson, Mississippi.

on the sand materials. Aggregate grading curves for these samples are presented in Figures 12-27. Atterberg limit values for clay materials are shown on the boring profiles (Figures 5-11).

Stability

Cross section

16. The cross section chosen for stability analysis (Figure 28) was obtained from data at sta 14+18 (DH-7). The profile consists of 6-1/2 ft of highly plastic, fat clay (CH) overlain by 20 ft of sand and silty soil overlain by 33-1/2 ft of garbage. Although not a typical or mean cross section, it was chosen as being critical due to the height of the slopes. The cross section shows some of the garbage behind the slope removed and backfilled with the low plastic clay (CL), silty sand (SM), and gravelly sand (SP) materials excavated from below since adequate compaction cannot be obtained on the landfill material. Soil parameters were estimated based on laboratory results and Standard Penetration Test (SPT) values.

17. Side slopes of 1V on 3H were chosen as the minimum grade on which construction and compaction equipment can operate safely and efficiently. Such equipment will be necessary in constructing the low permeability boundaries that are required.

Analysis

18. The design cross section was analyzed for stability using the wedge and arc methods. The wedge method was used to evaluate the upper failure plane along the lower surface of the garbage. The arc method was employed to check the stability throughout the entire cross section. Both analyses were performed for the case of sudden or rapid drawdown of the water level from el 170.0 to 137.0. It is anticipated that this case will be the most severe since rapid fluctuations in river stage are common along the Pearl. Figures 29 through 34 present results of the stability analyses. A minimum factor of safety of 1.20 was computed for an arc tangent to the upper surface of the CH material. The failure plane extends through the SP and SM-CL layers at a maximum depth of 5 ft.

Impervious Boundary

19. Environmental restraints possibly will dictate that measures be taken to prevent seepage of potentially hazardous leachate from the landfill into the river. If such restraints are required, a practical solution would be to blanket the sides of the channel with the CH material that will be excavated.

20. Eliminating the seepage problem with a low permeability blanket would create another problem. In the case of rapid drawdown, the hydrostatic pressures behind this blanket would be extremely high bringing about the possibility of blowouts in the clay liner. Some type of relief system, such as backdrains, seepage trenches, or relief wells, would be required to guard against failures of this nature. If environmental restrictions require that the bleedoff from such a relief system be treated, the more costly alternative of wells and pumps would be recommended to transport the contaminated material to holding ponds. Backdrains or trenches might be considered if the bleedoff could be allowed to seep back into the landfill or be discharged into the river.

21. With the significant design and construction effort to create a boundary between landfill and river, some type of protection against erosion of the clay liner should be considered. Alternatives would include chemical stabilization with lime or cement, fabric or membrane liners, and riprap. Riprap placed along the slopes beginning at the CH layer and extending upward along the slopes should prove to be the most reliable solution under these circumstances. Based on the results of tests performed locally on the banks of the Big Black River, a thickness of 12 to 18 in. is recommended. Locally available riprap will have a weight of approximately 165 lb/cu ft. Gradation limits are summarized in the following tabulation: *

| 12-in. Thickness | | 18-in. Thickness | |
|------------------------------|-------------------------------|------------------------------|-------------------------------|
| Percent Lighter by Weight | Limits of Stone Weight, lb | Percent Lighter by Weight | Limits of Stone Weight, lb |
| 100 | 35 - 86 | 100 | 117 - 292 |
| 50 | 17 - 26 | 50 | 58 - 86 |
| 15 | 5 - 13 | 15 | 18 - 43 |

* Office, Chief of Engineers, U. S. Army. 1971 (May). "Engineering and Design, Additional Guidance for Riprap Channel Protection," Engineer Technical Letter No. 1110-2-120, Washington, D. C.

Methane Gas

22. The presence of methane (natural gas) during excavation poses a potential safety hazard. If gas quantities are sufficient, large or concentrated, any open flame or spark could cause the gas to ignite. There have been incidents reported stating that diesel engines are difficult to stop if the air intake is rich in methane; however, most problems due to methane occur in situations where ventilation is poor. Measurements by the WES Environmental Laboratory show that the methane level at the site is considerable but probably will not be a permanent factor since the excavation will allow the gas to vent rapidly. The drilling crew reported no trouble with methane during the field exploration. However, the methane level should be monitored throughout the project to ensure the safety of construction workers.

Earthwork

23. Estimated quantities of earthwork for the proposed cutoff were calculated based on the channel design configuration (Figure 28). Six cross sections were chosen along the channel route at locations where boring data were available. Figures 35 through 40 show these sections and the areas used for the volume computations. Table 2 presents the computed volumes of cut in such a manner that the quantities of garbage, CH, CL, SM, and SP are readily distinguishable.

24. The amount of clay (CH) required for placement of a 5-ft-thick capping to a distance of 40 ft behind the slopes and for construction of a 3-ft-thick, low-permeability blanket over the slopes was calculated, as shown in Table 3. A shrinkage factor of 1.25 is assumed for the borrow material. Based on these calculations, a quantity of 2000 cu yd of CH soil would be required from some other source.

25. The amount of backfill required to replace the garbage immediately behind the slopes was computed, and the results are listed in Table 4. A shrinkage factor of 1.25 is assumed for the borrow material. These calculations show that a sufficient quantity of CL, SM, and SP materials will be excavated to meet the backfill demand.

26. The required volume of riprap to provide an 18-in.-thick protective layer beginning at the heavy clay and extending upward along the length of the side slopes was calculated, as shown in Table 5.

Summary

27. The proposed channel bottom, el 227.0, will be in Yazoo clay, except at boring DH-6 where it will be in alluvial sand. The Yazoo clay is 2 to 3 ft below the proposed channel bottom at boring DH-6. A typical vertical section along the proposed channel cut will have 5 to 10 ft of Yazoo clay overlain by 20 to 25 ft of alluvium, which is predominantly fine-to-medium sand, overlain by up to 30 ft of landfill material. Yazoo clay excavated from the channel cut is the only suitable material at the site that can be used for the upper bank blanket.

Recommended Procedures

28. The design procedures recommended as a result of the investigation are as follows:

- a. Side slopes of 1V on 3H are recommended to accommodate construction and compaction equipment. Preliminary stability analyses yielded a minimum factor of safety of 1.20.
- b. An estimated 573,000 cu yd of landfill material must be removed to another location or placed back on the landfill and covered. The quantity of cover material will be dependent on the desired cover ratio.
- c. If environmental restraints require the minimization of leachate infiltration into the river, the placement of a 3-ft-thick (minimum) CH liner along the side slopes is recommended.
- d. If the cutoff design includes a low-permeability liner along the sides, a relief system should be designed to relieve the hydrostatic pressures during periods of rapid drawdown. The type of system will be dependent on environmental considerations.
- e. Riprap along the side slopes is recommended to prevent erosion of

the clay blanket. An 18-in.-thickness of riprap on the slopes would require 20,000 cu yd of stone.

- f. Methane gas levels should be monitored throughout the excavation. It is anticipated that the gas will vent quickly and cause no major permanent problems.
- g. The earthwork quantities shown in Tables 2 through 4 should be used in the cost estimate. A total volume of 830,000 cu yd is to be excavated. Of this total, 80,000 cu yd are CH, 177,000 cu yd are CL, SM, and SP soils, and 573,000 cu yd are landfill. The excavated CH material will be used to construct the clay blanket (50,000 cu yd) and cap behind the slopes (32,000 cu yd). An additional 2,000 cu yd of CH material must be acquired from another source should the liner be included in the design. The excavated CL, SM, and SP soils will be used to backfill behind the slopes (150,000 cu yd). An excess of 27,000 cu yd will be available as possible cover material for garbage.

Table 1
Stratigraphic Column*

| Era | | Cenozoic | | Tertiary | | Quaternary | | System | | Series | | Group | | Formation | | Description | | | |
|--------|--|----------|--|----------|--|------------|--|---------|----------------|------------|--------|-----------|--|-----------|--|-------------|--|--|--|
| Upper | | | | | | | | Jackson | Moody's Branch | Yazoo Clay | | | | | | | | | |
| Middle | | | | | | | | | | | Eocene | | | | | | | | |
| Lower | | | | | | | | | | | | Claiborne | | | | | | | |
| | | | | | | | | | | | | Cockfield | | | | | | | |
| | | | | | | | | | | | | | | | | | | | |

* Modified from: Moore, W. H. 1965. "Hinds County Geology and Mineral Resources," Bulletin 105, Mississippi Geological Economic and Topographical Survey.

Table 2
Estimated Volume of Earthwork for Proposed Channel Cut

| Station ft | CH | CL, SM, and SP | End Area, sq ft | End Area sq ft | Distance ft | Volume of Cut, cu yd | | | Total Volume cu yd | Accumulated Total Volume cu yd |
|---------------|-------|----------------|-----------------|-------------------|----------------|----------------------|----------------|---------|-----------------------|--------------------------------------|
| | | | | | | CH | CL, SM, and SP | Garbage | | |
| 0+00 | 1,152 | 3,859 | 9,219 | 14,230 | 424 | 19,300 | 43,052 | 154,666 | 217,017 | 217,017 |
| 4+24 | 1,306 | 1,624 | 10,479 | 13,409 | 334 | 18,259 | 26,132 | 108,346 | 152,737 | 369,754 |
| 7+58 | 1,646 | 2,601 | 7,038 | 11,285 | 277 | 19,252 | 21,750 | 54,820 | 95,822 | 465,576 |
| 10+35 | 2,107 | 1,639 | 3,649 | 7,395 | 447 | 19,312 | 49,840 | 159,935 | 229,087 | 694,663 |
| 14+82 | 226 | 4,382 | 15,672 | 20,280 | 248 | 4,345 | 36,552 | 94,538 | 135,435 | |
| 17+30 | 720 | 3,577 | 4,913 | 9,210 | | | | | | |
| | | | | | | 80,468 | 177,362 | | 572,305 | |

Table 3
Required Volume of CH Material

| Station ft | Required Length ft | Distance ft | Average Required Length, ft | Surface Area sq ft | Volume cu ft | Volume cu yd | Accumulated Volume cu yd | Total cu yd |
|---------------|--------------------------|----------------|-----------------------------------|--------------------------|-----------------|-----------------|--------------------------------|-----------------------|
| 0+00 | 244 | 424 | 228.5 | 96,884 | 290,652 | 10,765 | | |
| 4+24 | 213 | 334 | 119 | 66,466 | 199,398 | 7,386 | 19,765 | |
| 7+58 | 185 | 277 | 147.5 | 40,858 | 122,573 | 4,540 | 18,151 | |
| 10+35 | 110 | 447 | 226.5 | 101,246 | 303,737 | 11,250 | 22,691 | |
| 14+82 | 343 | 248 | 235.5 | 58,404 | 175,212 | 6,489 | 33,941 | |
| 17+30 | 128 | | | | | | 40,430 | |
| | | | | | | | | (for 3-ft clay liner) |

Required volume of CH = 40,430 cu yd (*1.25) = 50,538 cu yd
for impermeable liner

Required volume of CH = 25,630 cu yd (*1.25) = 32,037 cu yd
for 5-ft capping
behind slopes

Total CH required = 82,575 cu yd

CH excavated = 80,468 cu yd

CH to be acquired
from other source = 2,107 cu yd

* Shrinkage factor.

Table 4
Required Volume of CL, SM, and SP Soils for Backfill

| Station ft | Area sq ft | Average | | | Volume cu yd | Accumulated Volume cu yd | Total |
|---------------|---------------|---------------|----------------|-----------------|-----------------|--------------------------------|-------|
| | | Area sq ft | Distance ft | Volume cu yd | | | |
| 0+00 | 1,763 | | 2,127 | 424 | 33,402 | | |
| 4+24 | 2,490 | | 1,887 | 334 | 23,343 | 33,402 | |
| 7+58 | 1,283 | | 913 | 277 | 9,367 | 56,745 | |
| 10+35 | 543 | | 2,148 | 447 | 35,562 | 66,112 | |
| 14+82 | 3,752 | | 1,951 | 248 | 17,921 | 101,674 | |
| 17+30 | 149 | | | | | 119,595 | |

Required CL, SM, and SP
 for backfill = 119,595 (*1.25) = 149,494 cu yd

Excavated CL, SM, and SP = 177,326 cu yd

Excess CL, SM, and SP = 27,832 cu yd

* Shrinkage factor.

Table 5
Volume of Riprap Required for 18-in. Layer

| Station ft | Required Length ft | Distance ft | Average Required Length ft | Surface Area sq ft | Volume cu ft | Volume cu yd | Accumulated Total Volume cu yd |
|---------------|--------------------------|----------------|----------------------------------|--------------------------|-----------------|-----------------|--------------------------------------|
| 0+00 | 244 | 424 | 228.5 | 96,884 | 145,326 | 5,383 | |
| 4+24 | 213 | 334 | 199 | 66,466 | 99,699 | 3,693 | 5,383 |
| 7+58 | 185 | 277 | 147.5 | 40,858 | 61,287 | 2,270 | 9,076 |
| 10+35 | 110 | 447 | 226.5 | 101,246 | 151,869 | 5,625 | 11,346 |
| 14+82 | 343 | 248 | 235.5 | 58,404 | 87,606 | 3,245 | 16,971 |
| 17+30 | 128 | | | | | | 20,216 |

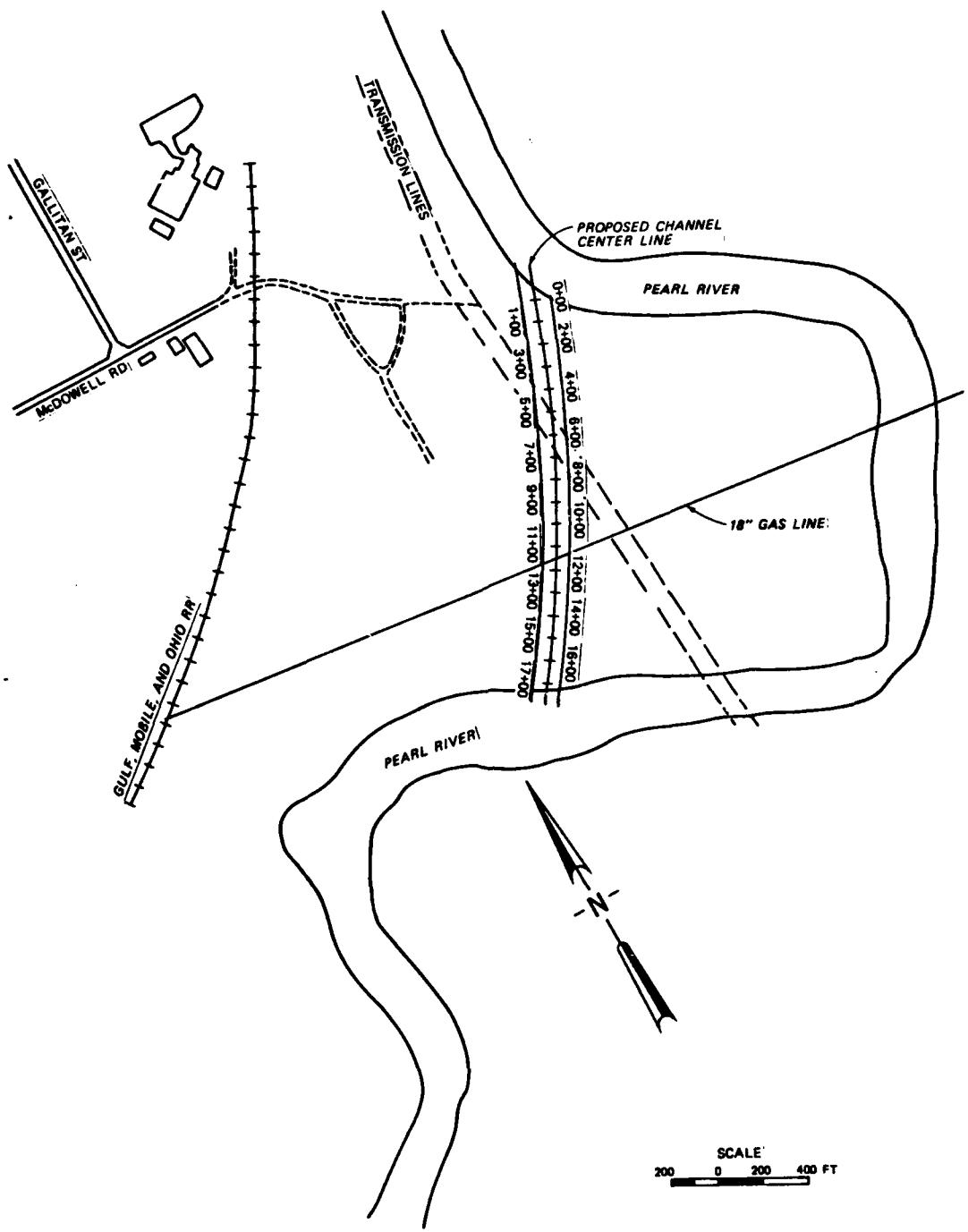


Figure 1. Location sketch with proposed cutoff marked in 100-ft increments

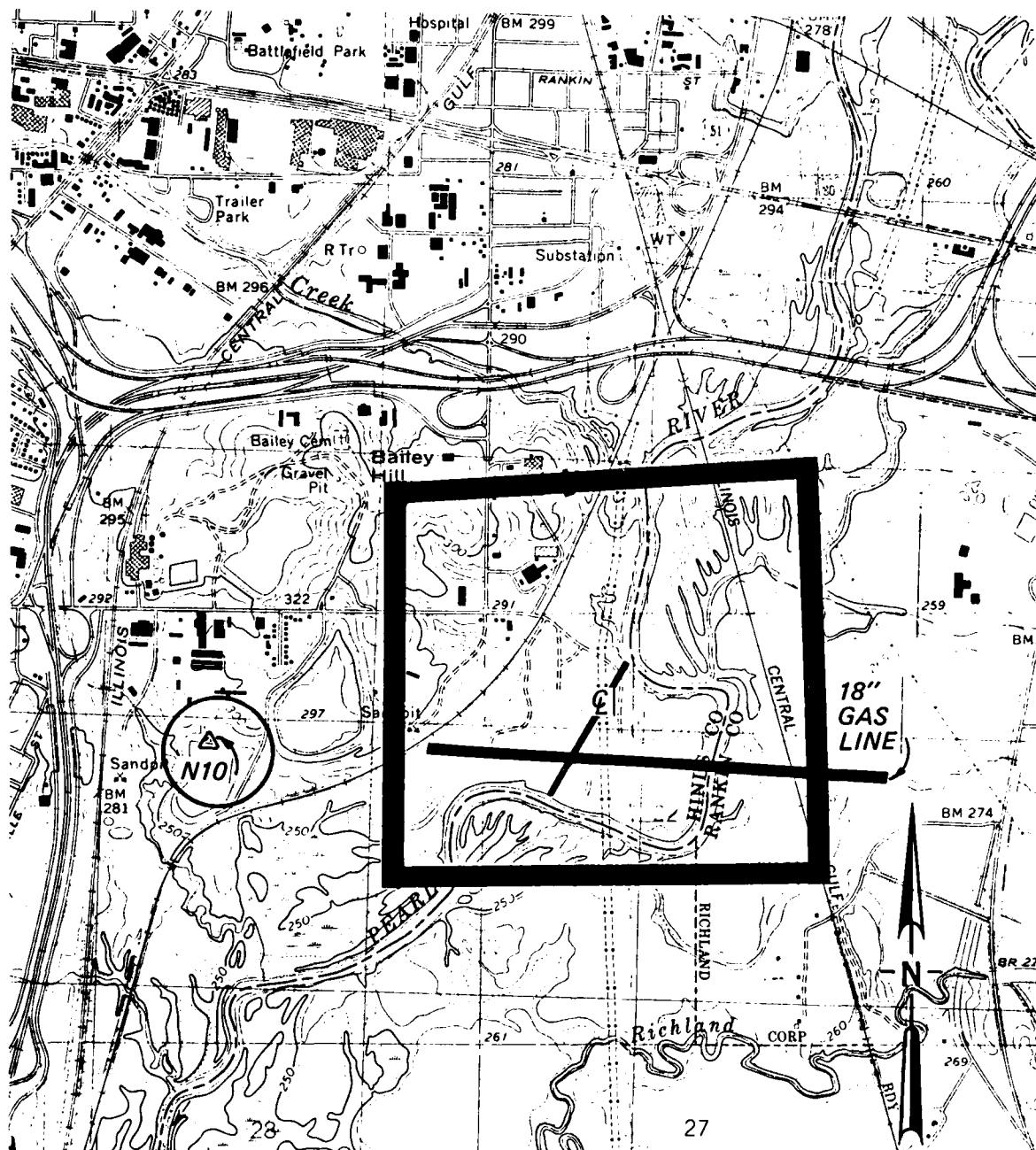


Figure 2. Location map with the center line of the proposed channel (Mississippi Geological Survey file boring N10 located west of the site)

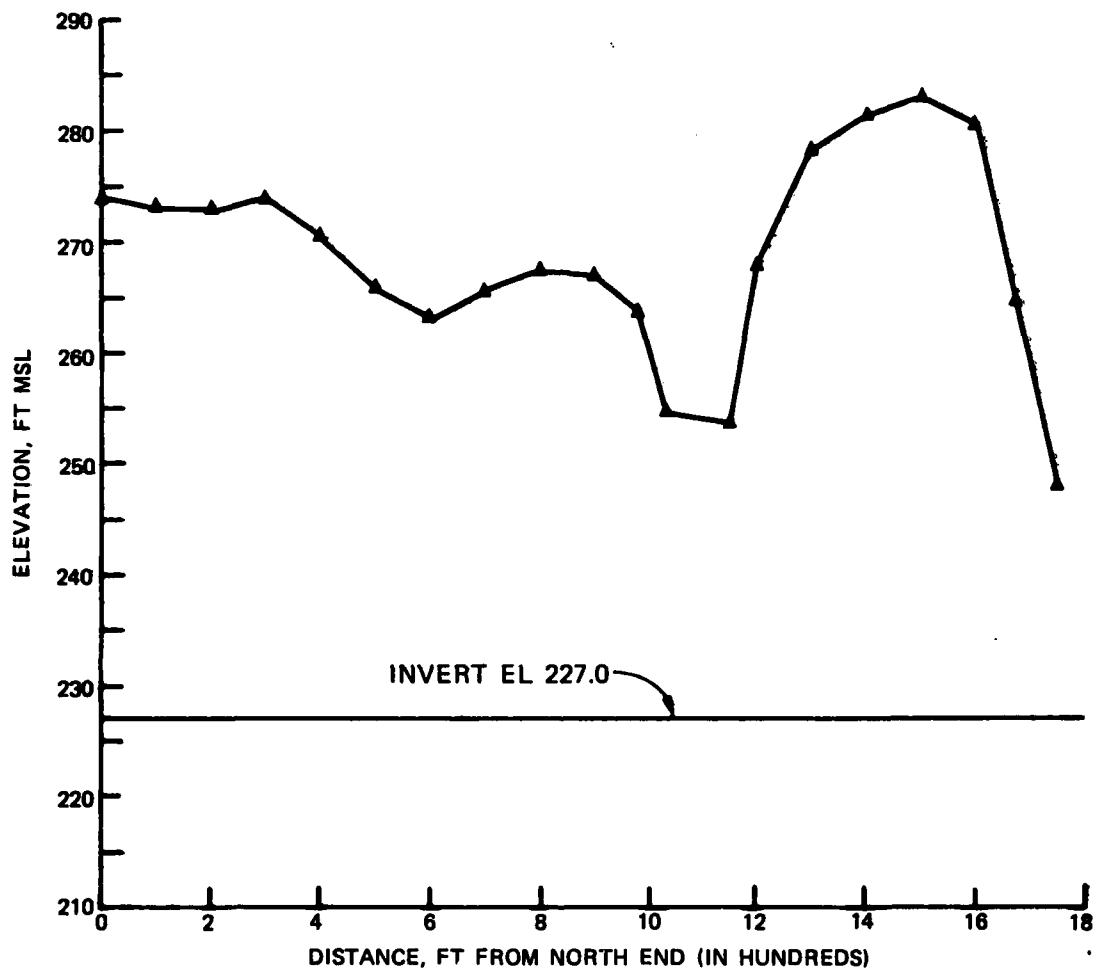


Figure 3. Center-line profile along proposed channel route

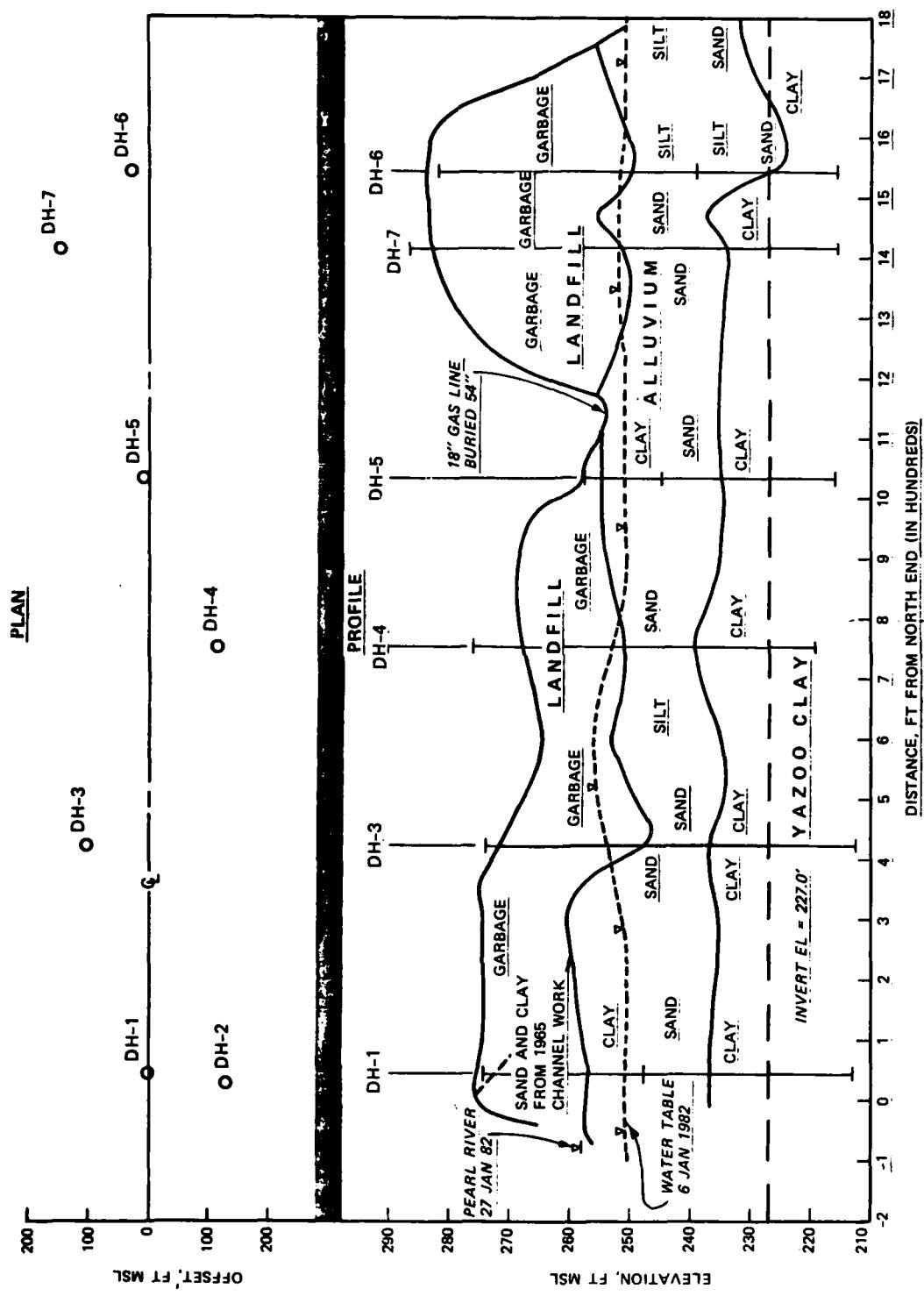


Figure 4. Plan view showing splitspoon sampling sites and the profile resulting from the subsurface investigation

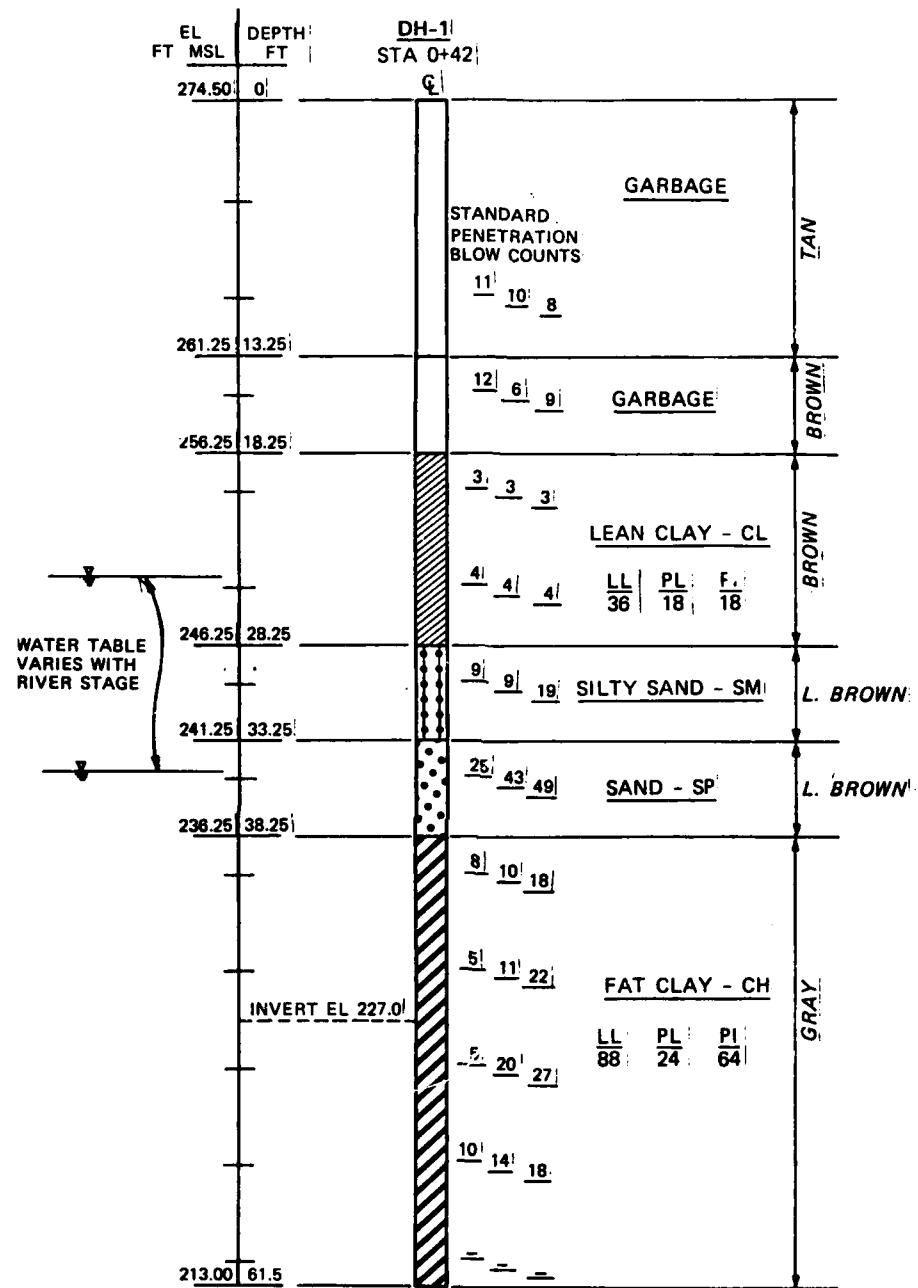


Figure 5. Boring profile from sample site DH-1

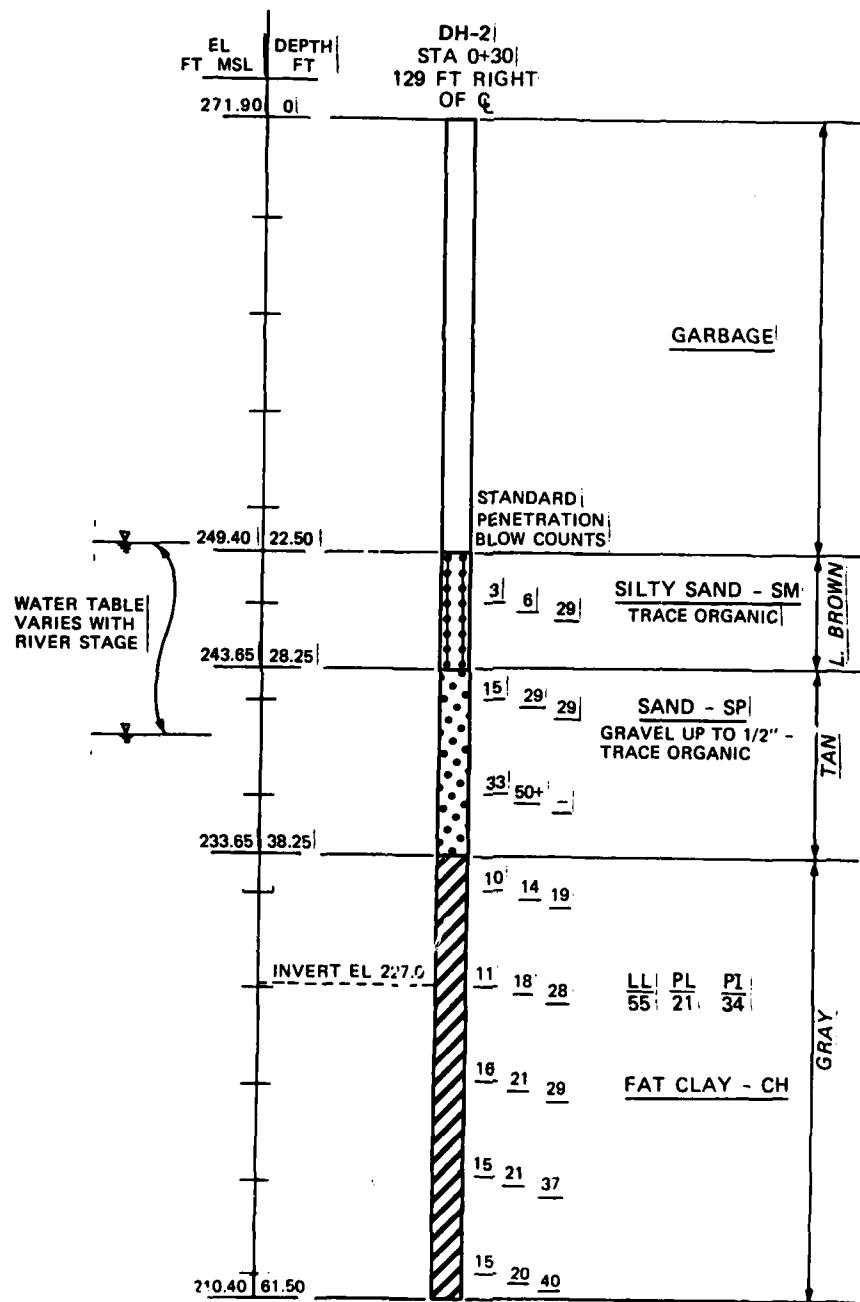


Figure 6. Boring profile from sample site DH-2

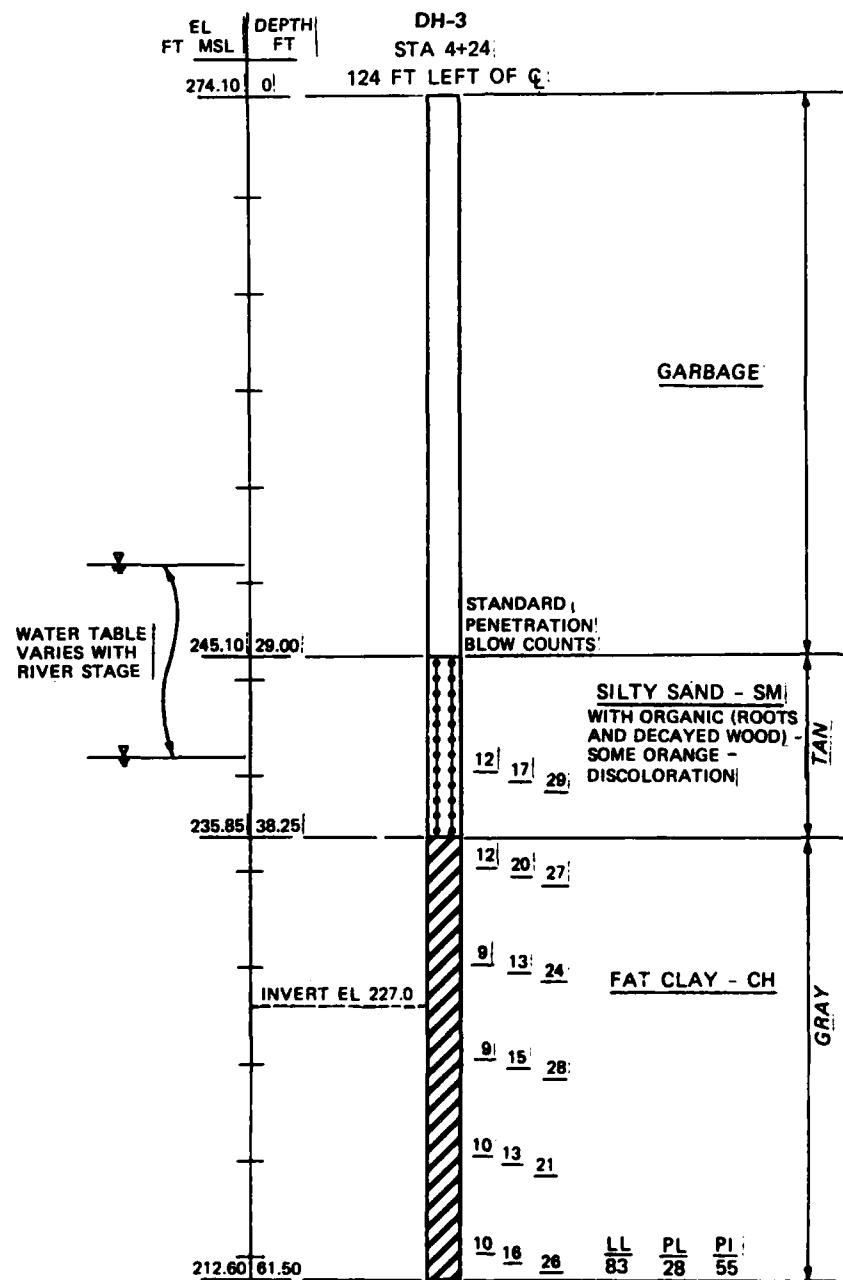


Figure 7. Boring profile from sample site DH-3

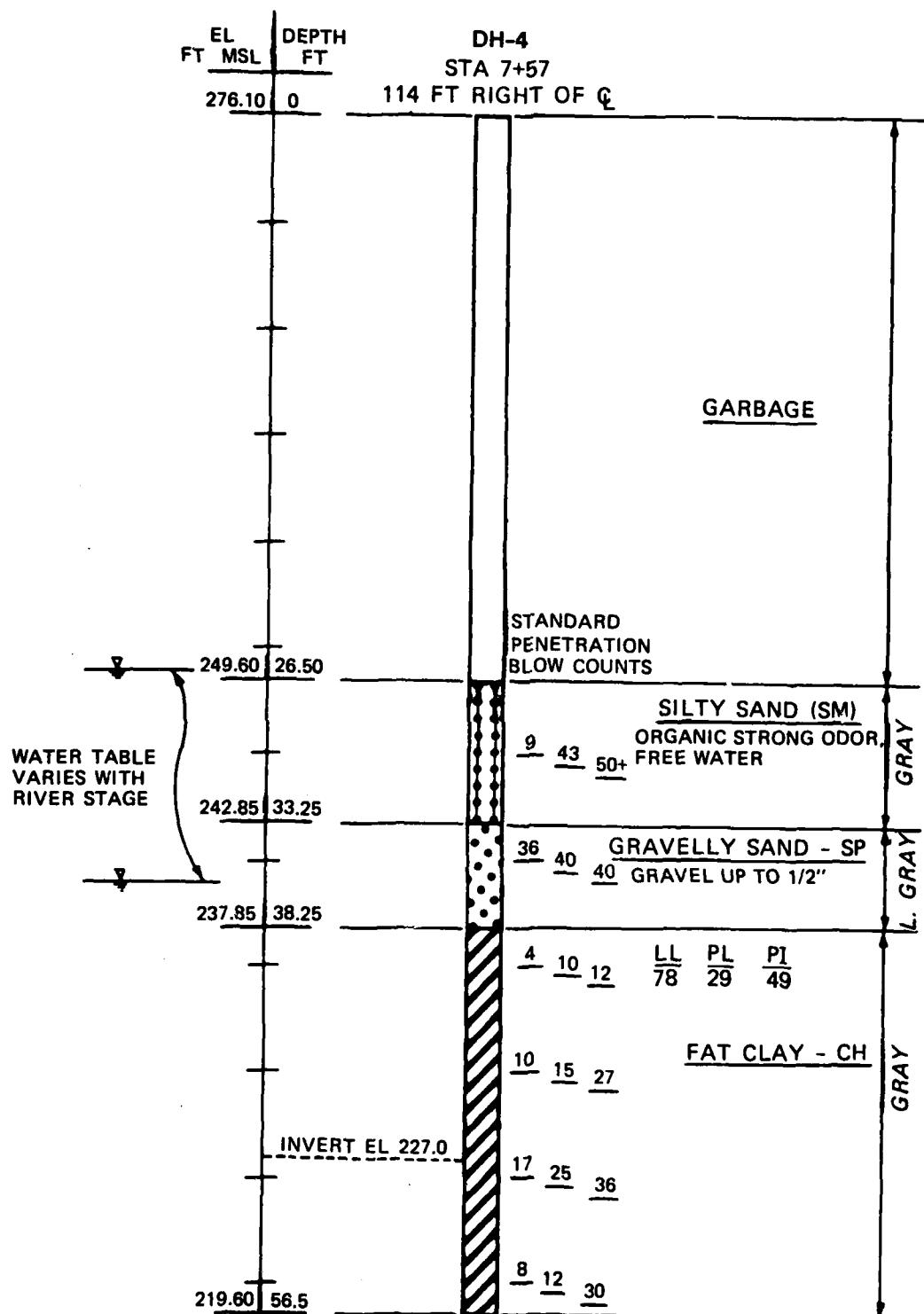


Figure 8. Boring profile from sample site DH-4

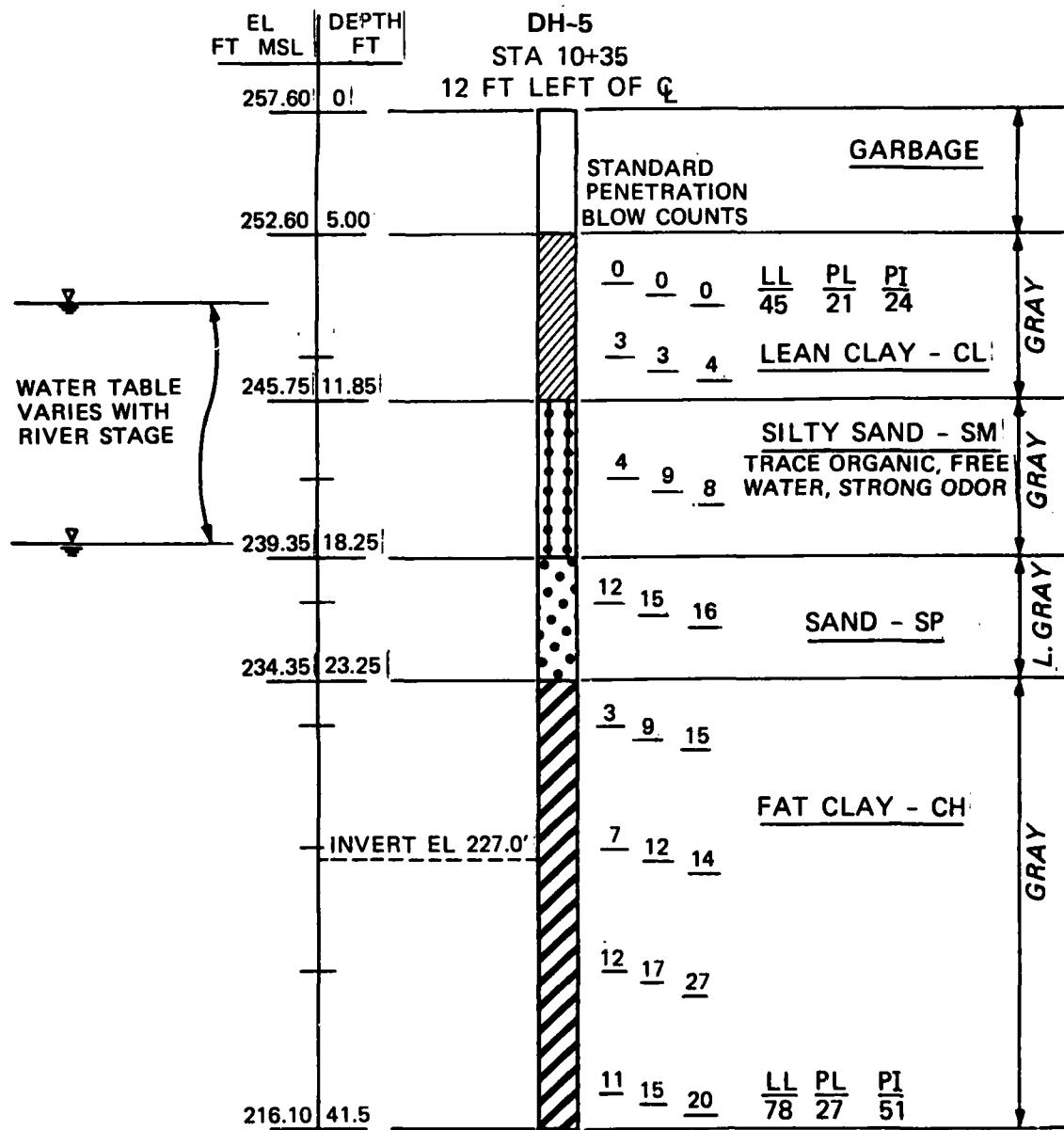


Figure 9. Boring profile from sample site DH-5

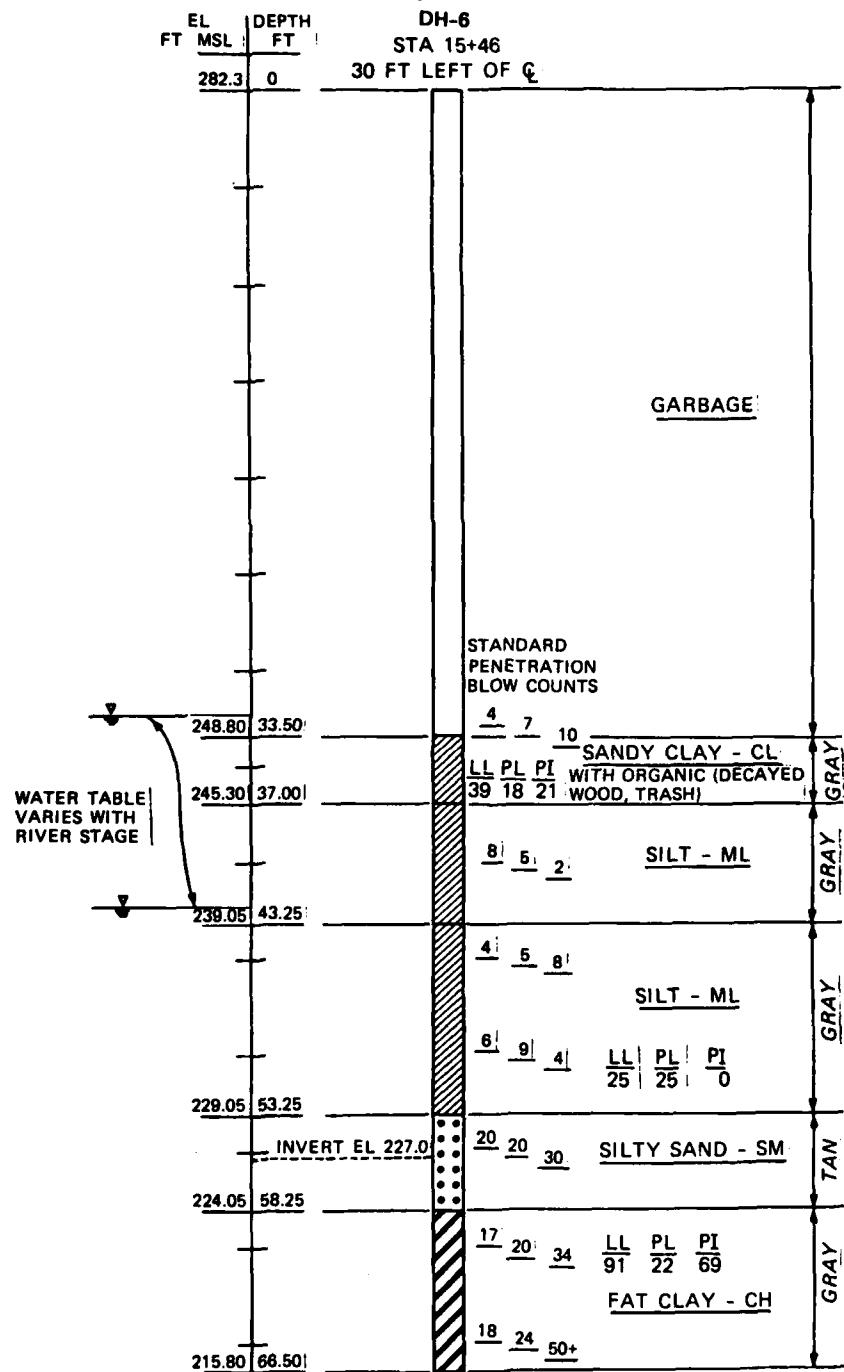


Figure 10. Boring profile from sample site DH-6

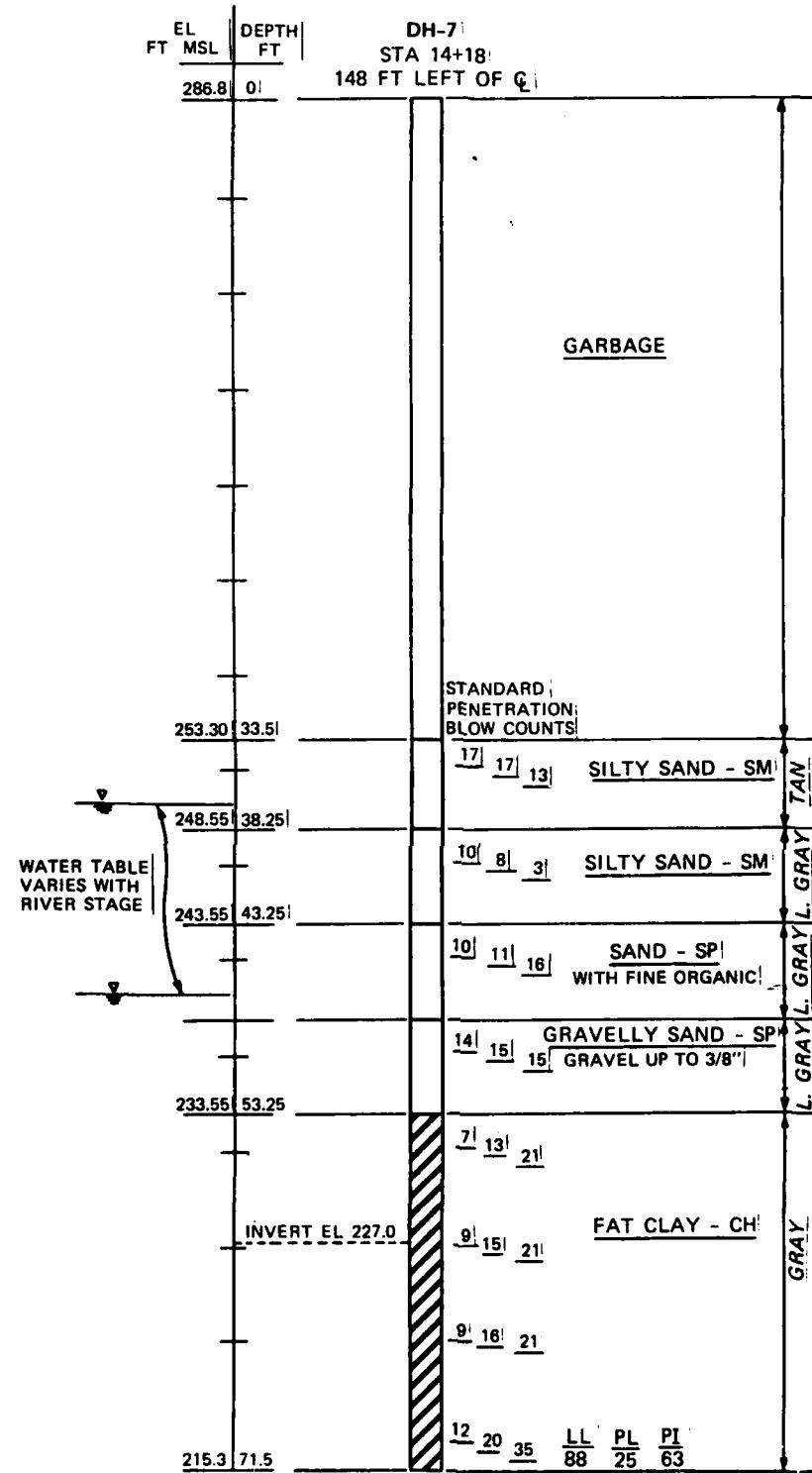


Figure 11. Boring profile from sample site DH-7

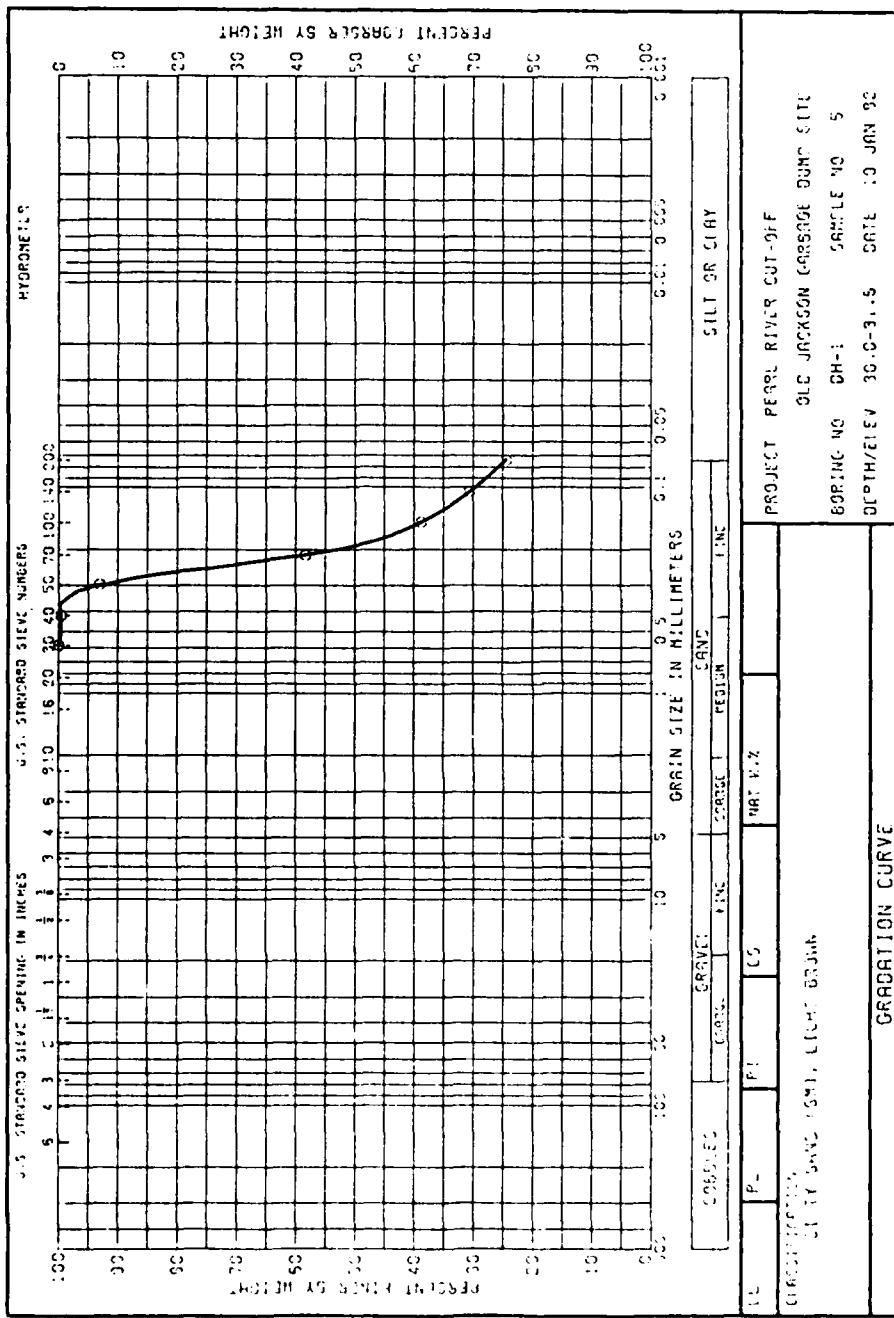


Figure 12. Aggregate grading curve for boring DH-1, depth 30.0 - 31.5

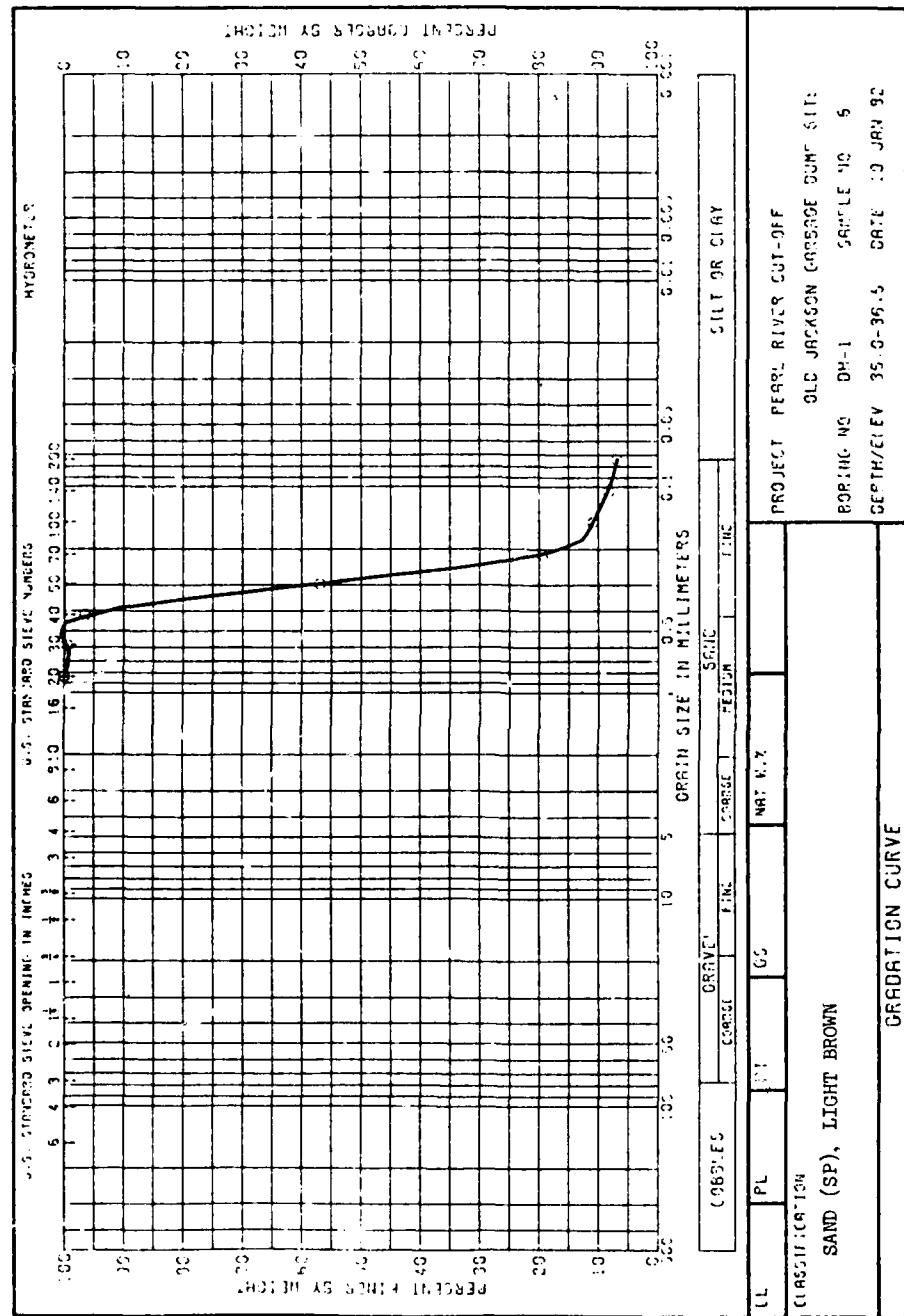


Figure 13. Aggregate grading curve for boring DH-1, depth 35.0 - 36.5

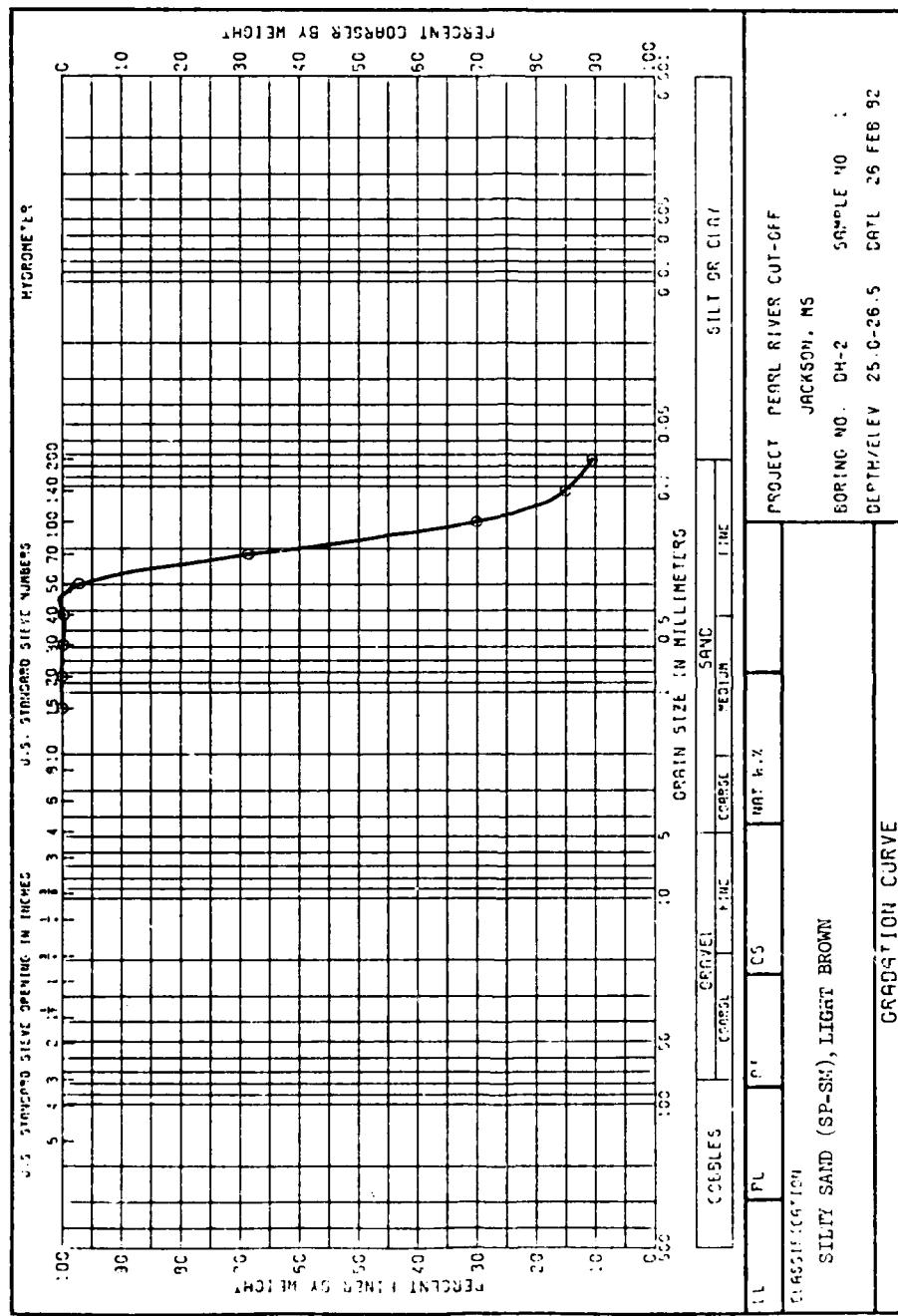


Figure 14. Aggregate grading curve for boring DH-2, d_{depth} 25.0 - 26.5

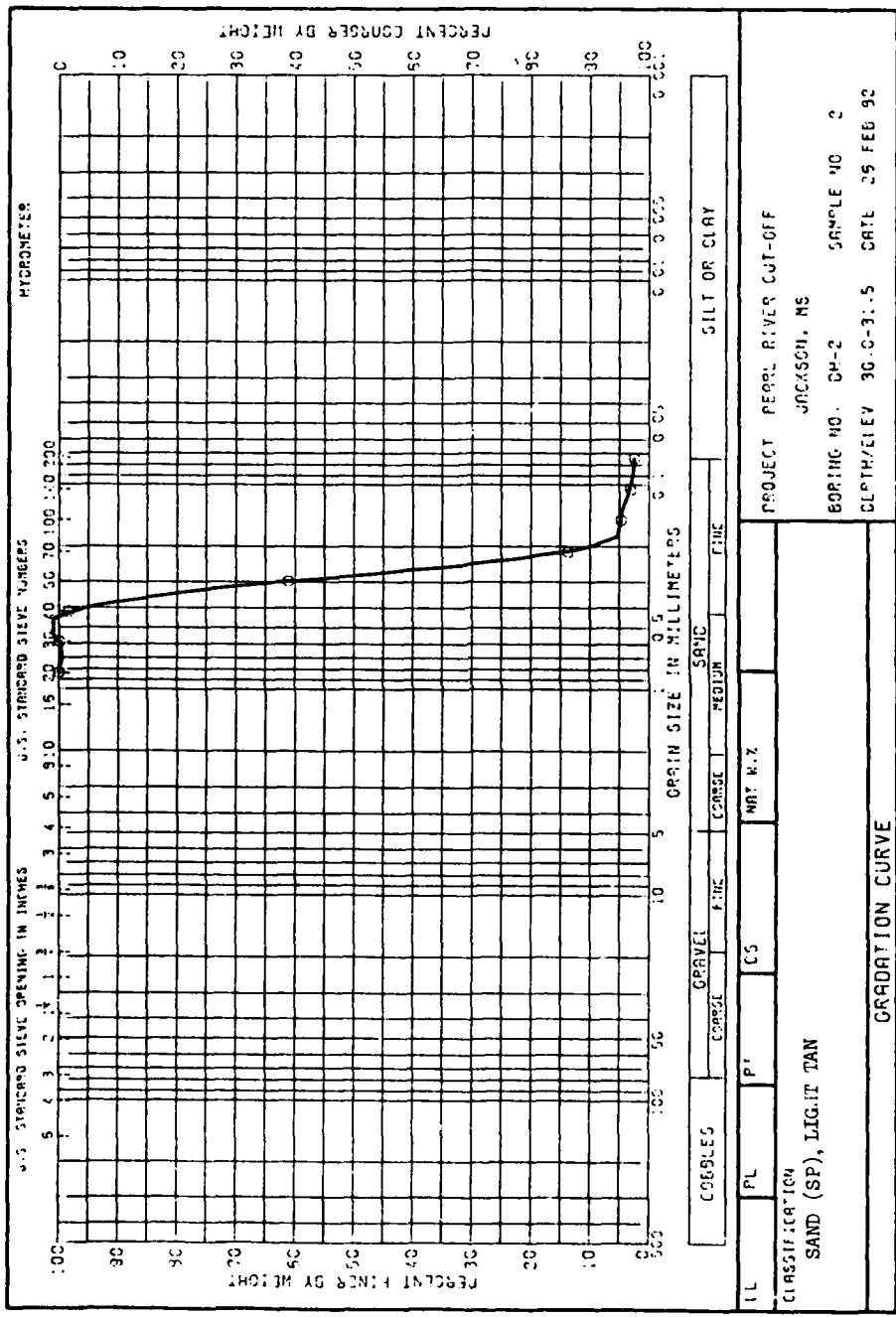
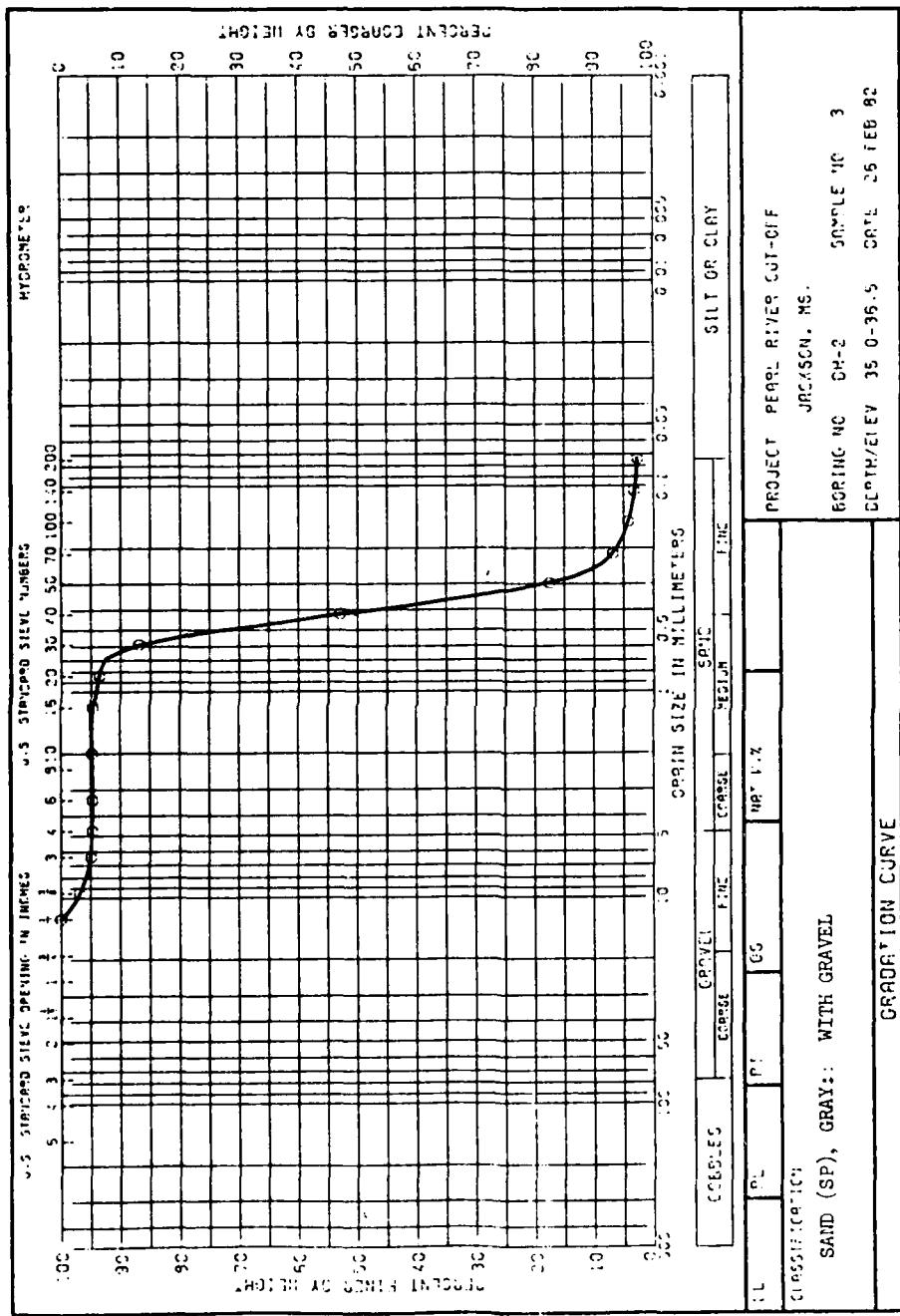


Figure 15. Aggregate grading curve for boring DH-2, depth 30.0 - 31.5



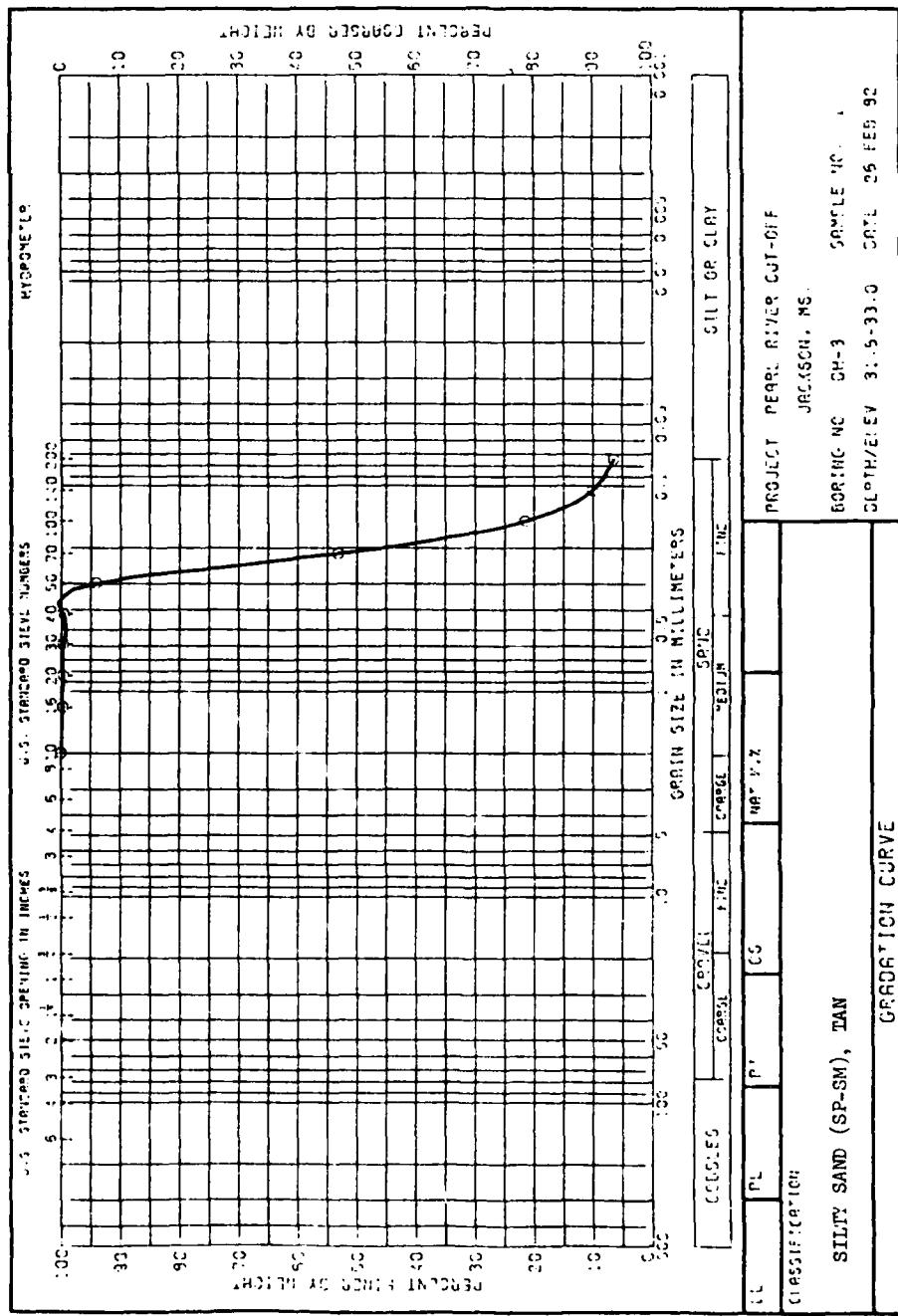
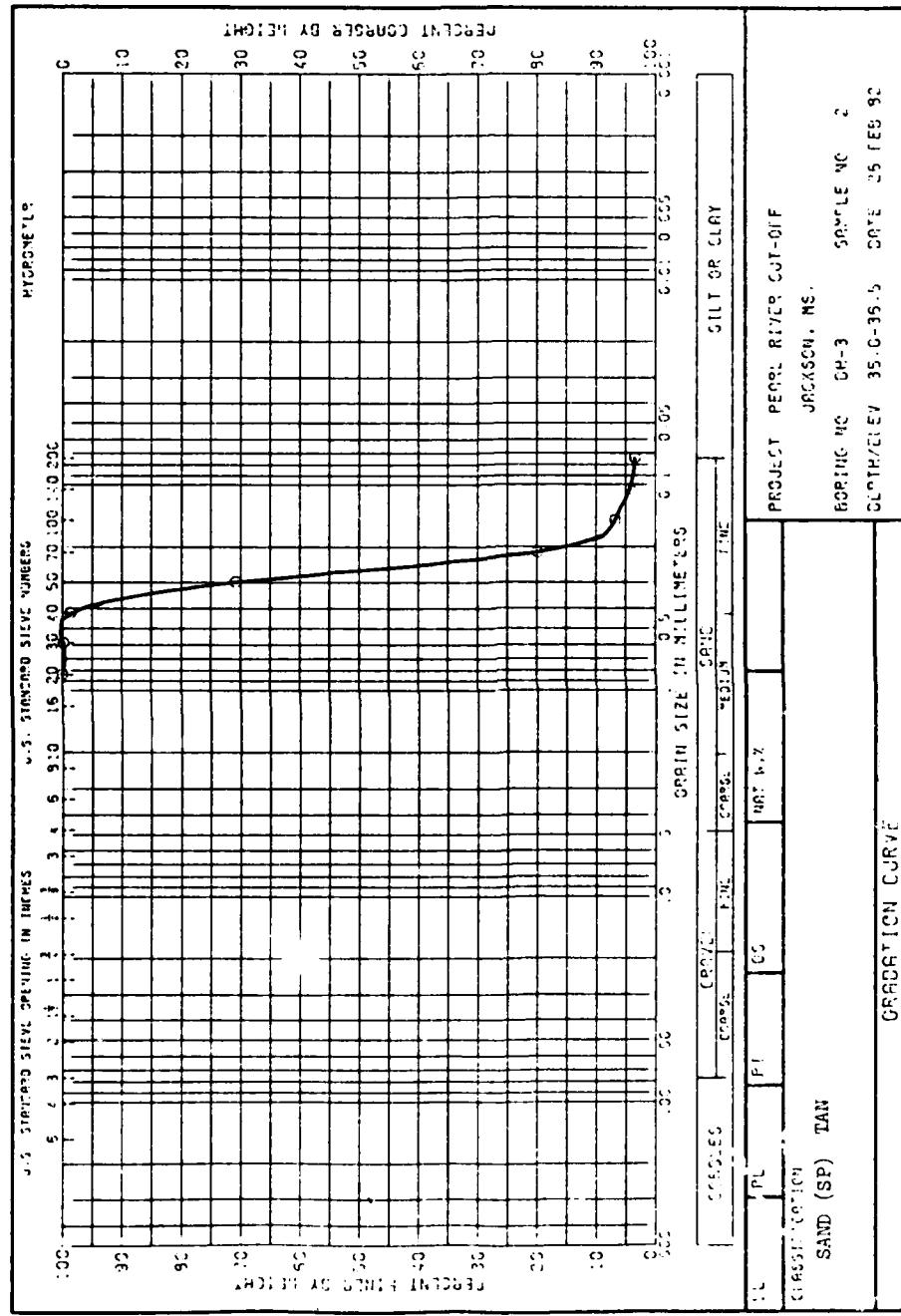


Figure 17. Aggregate grading curve for boring DH-3, depth 31.5 - 33.0



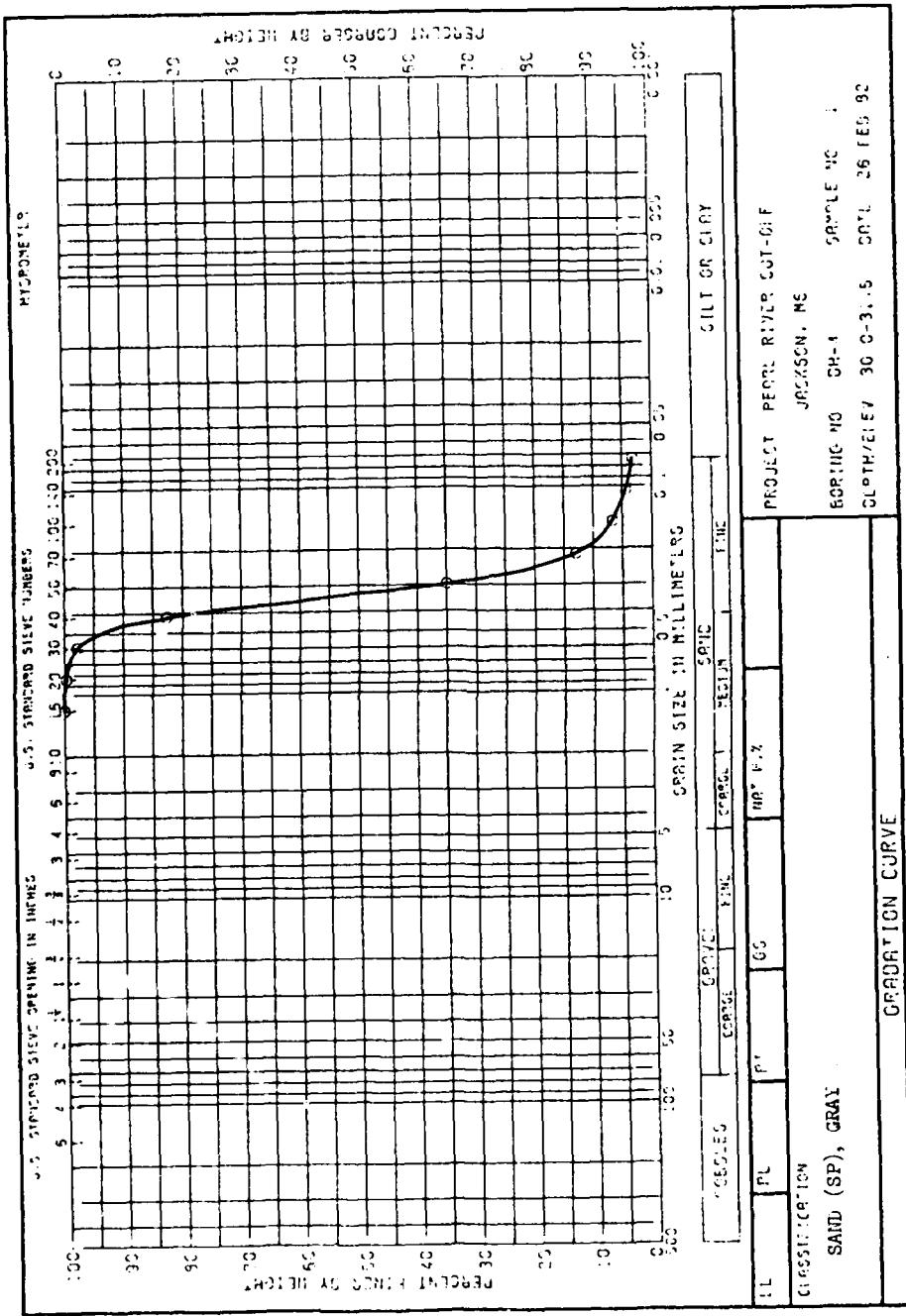


Figure 19. Aggregate grading curve for boring DH-4, depth 30.0 - 31.5

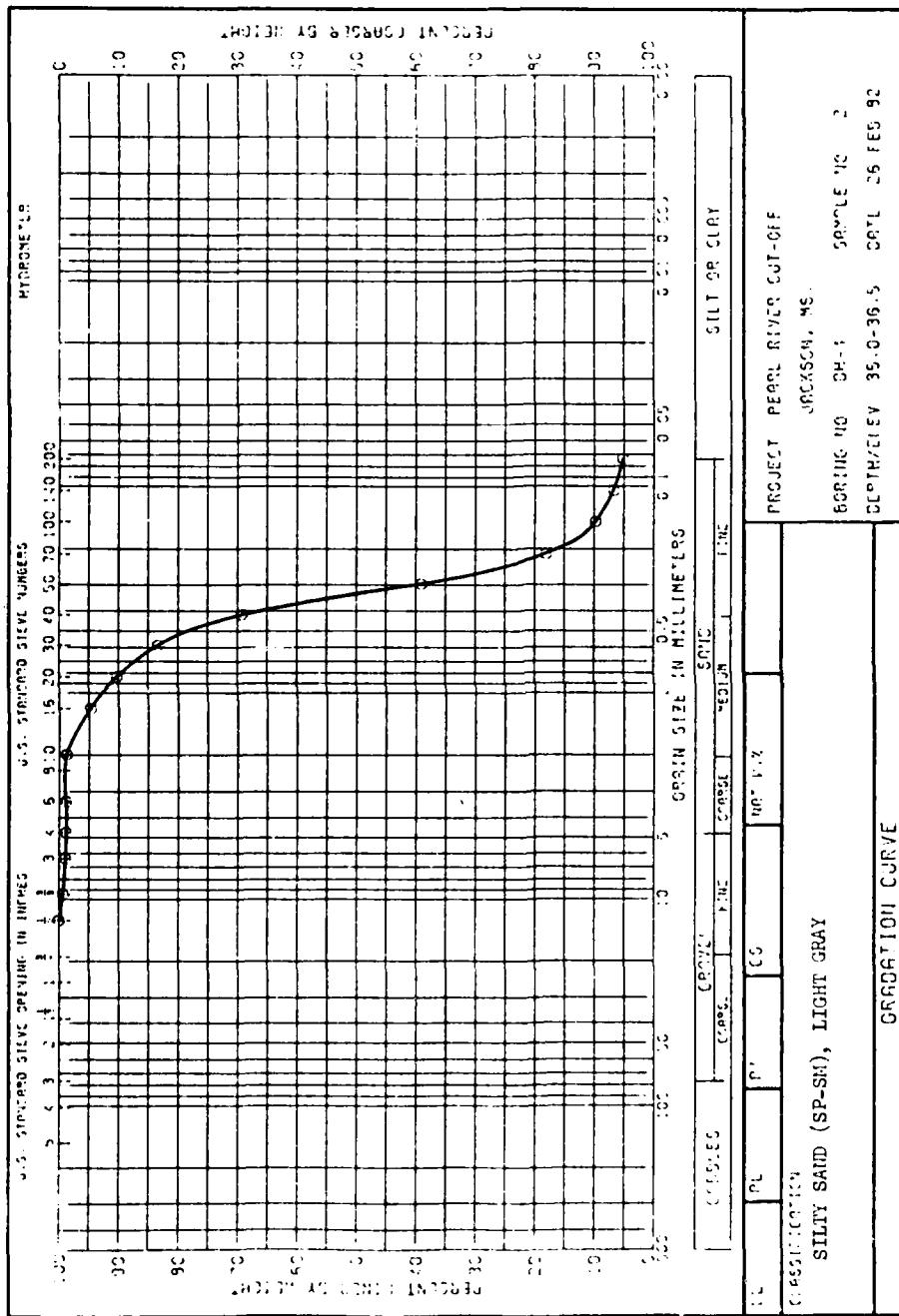


Figure 20. Aggregate grading curve for boring DH-4, depth 35.0 - 36.5

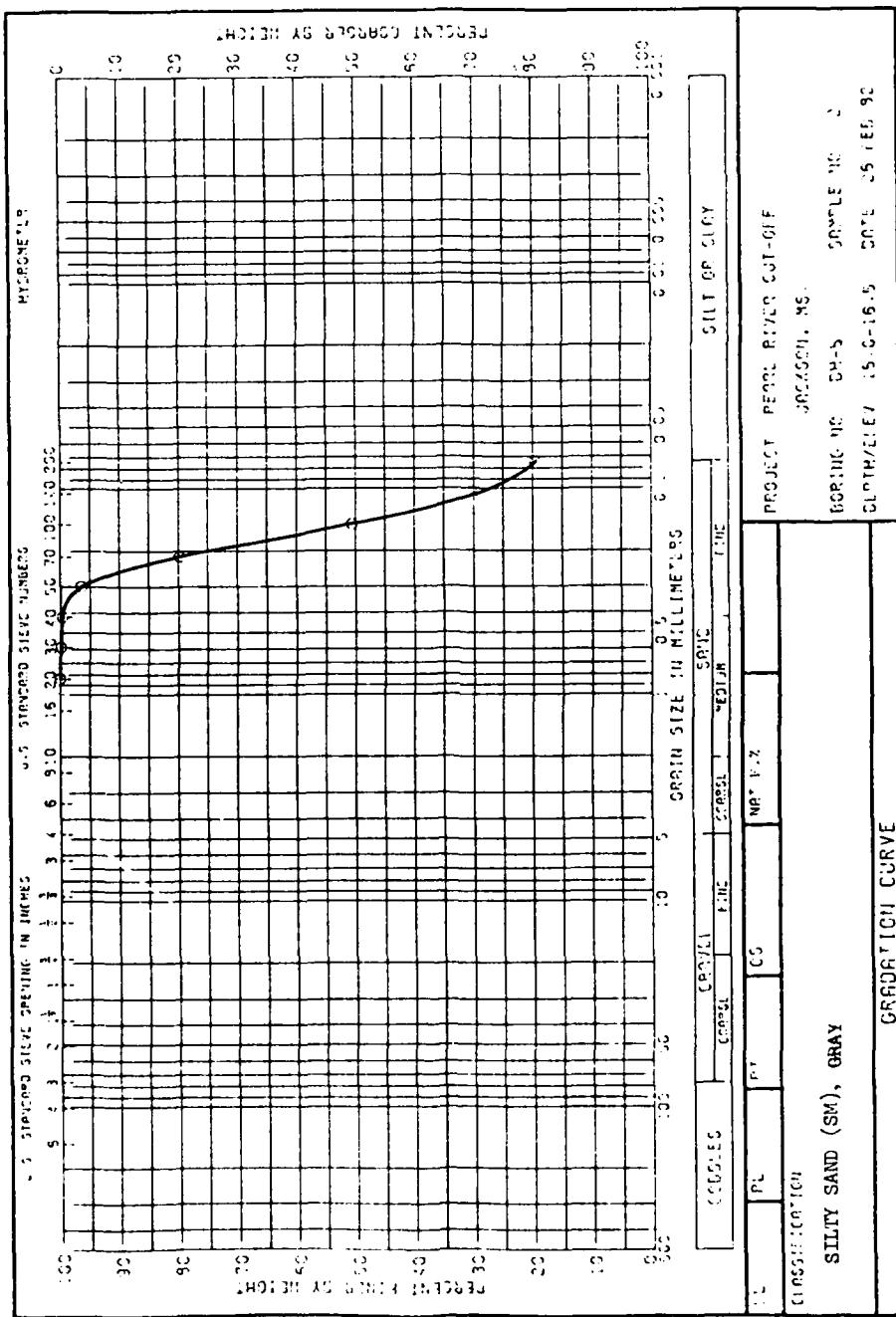


Figure 21. Aggregate grading curve for boring DH-5, depth 15.0 - 16.5

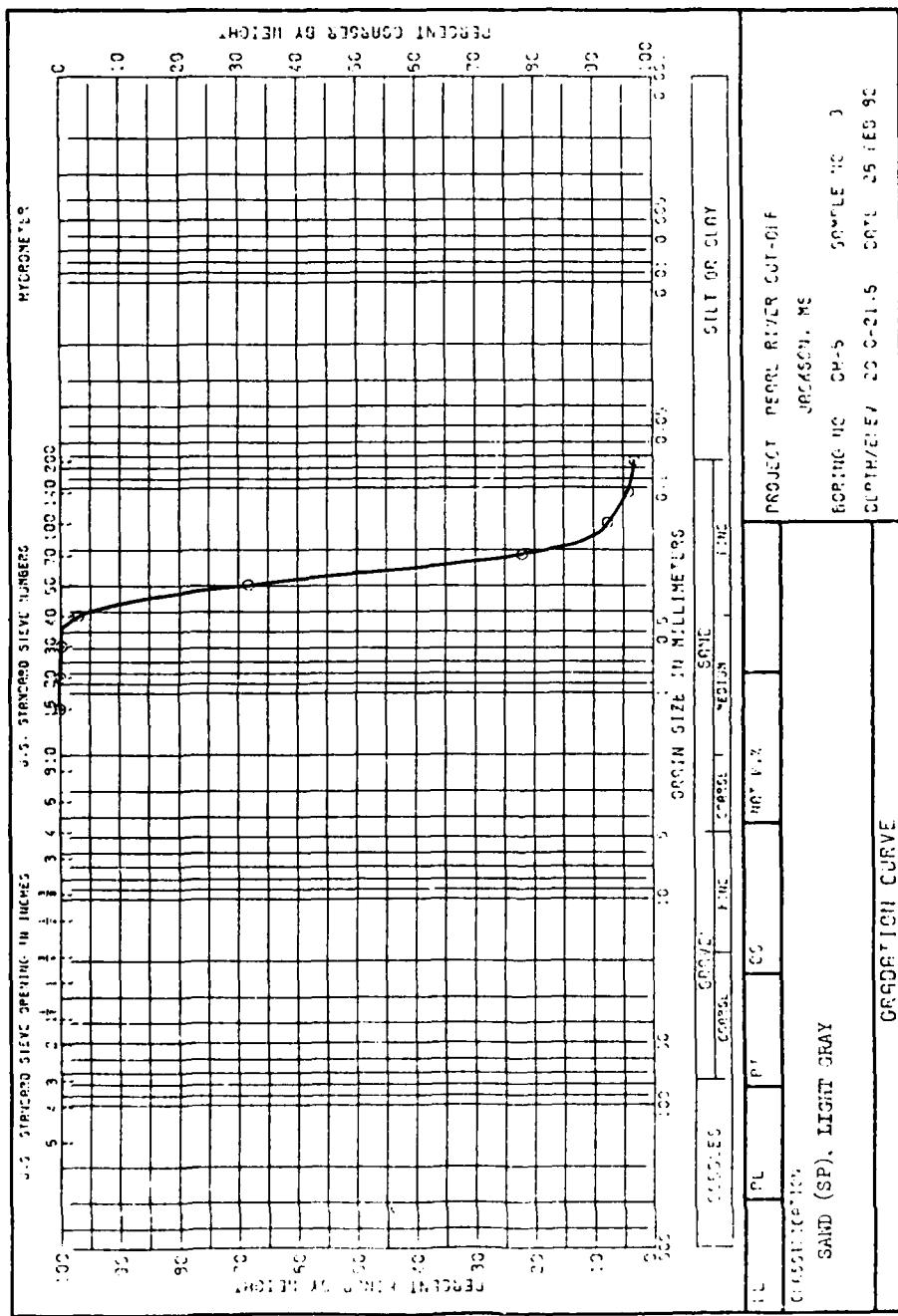


Figure 22. Aggregate grading curve for boring DH-5, depth 20.0 - 21.5

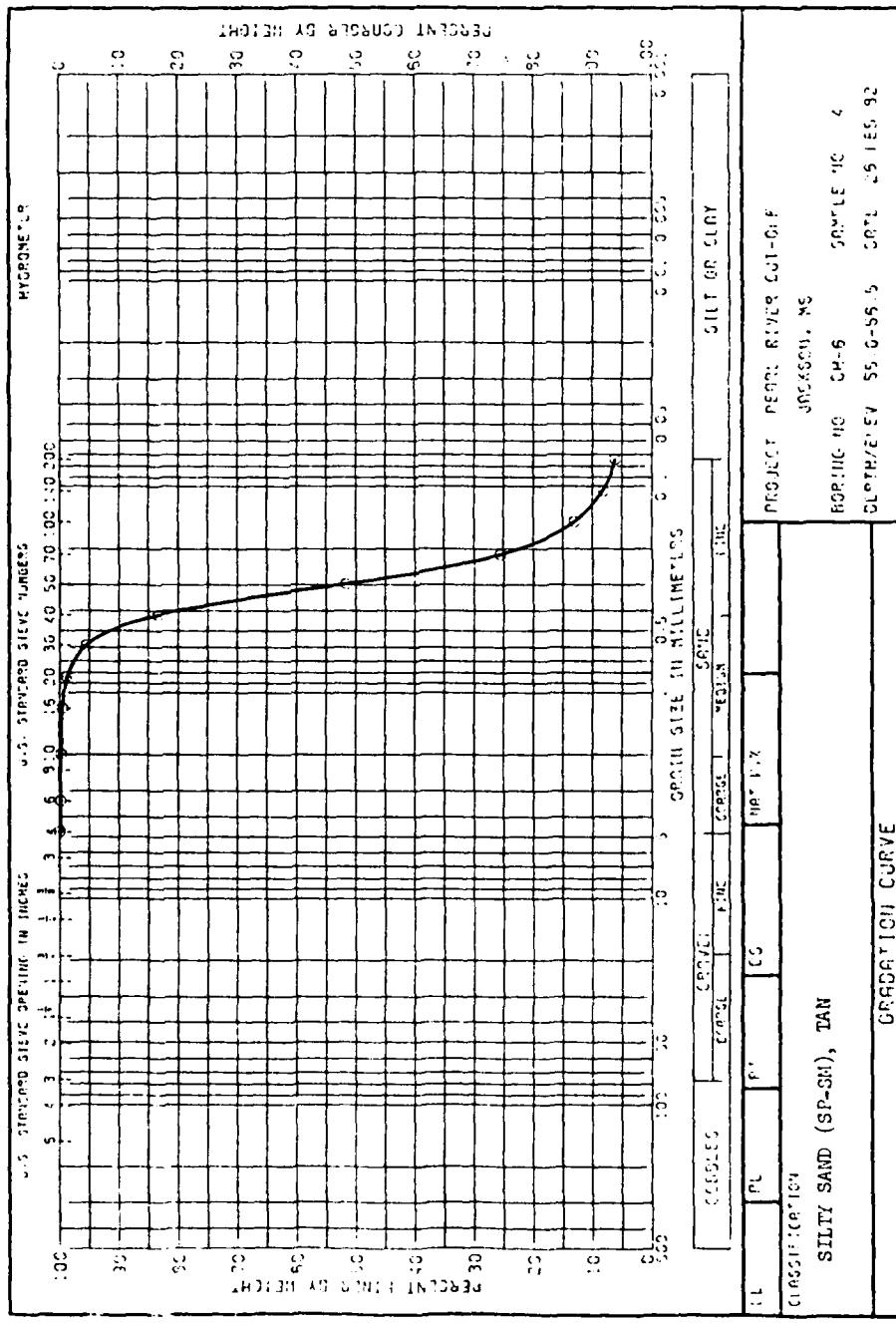


Figure 23. Aggregate grading curve for boring DH-6, depth 55.0 - 56.5

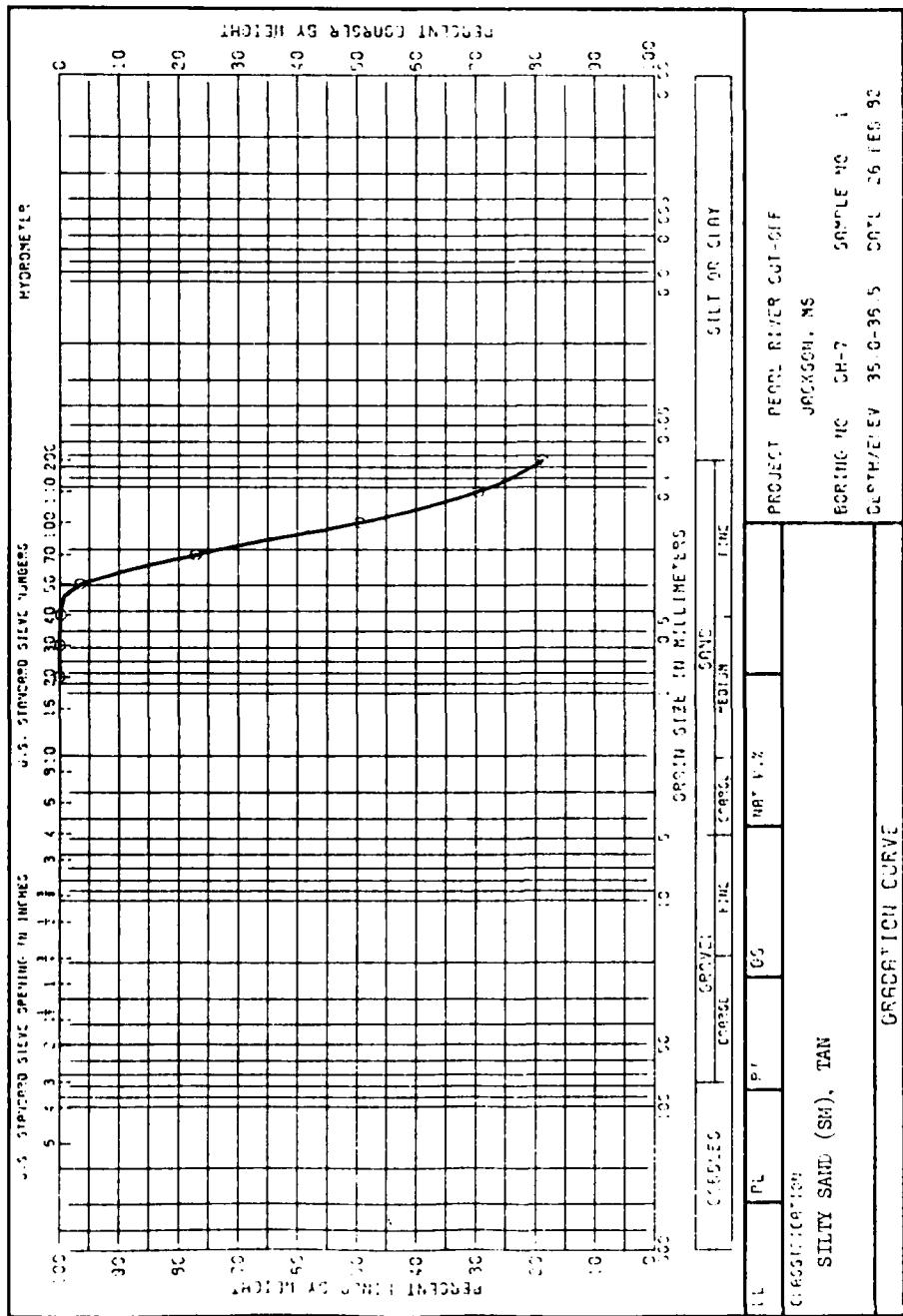


Figure 24. Aggregate grading curve for boring DH-7, depth 35.0 - 36.5

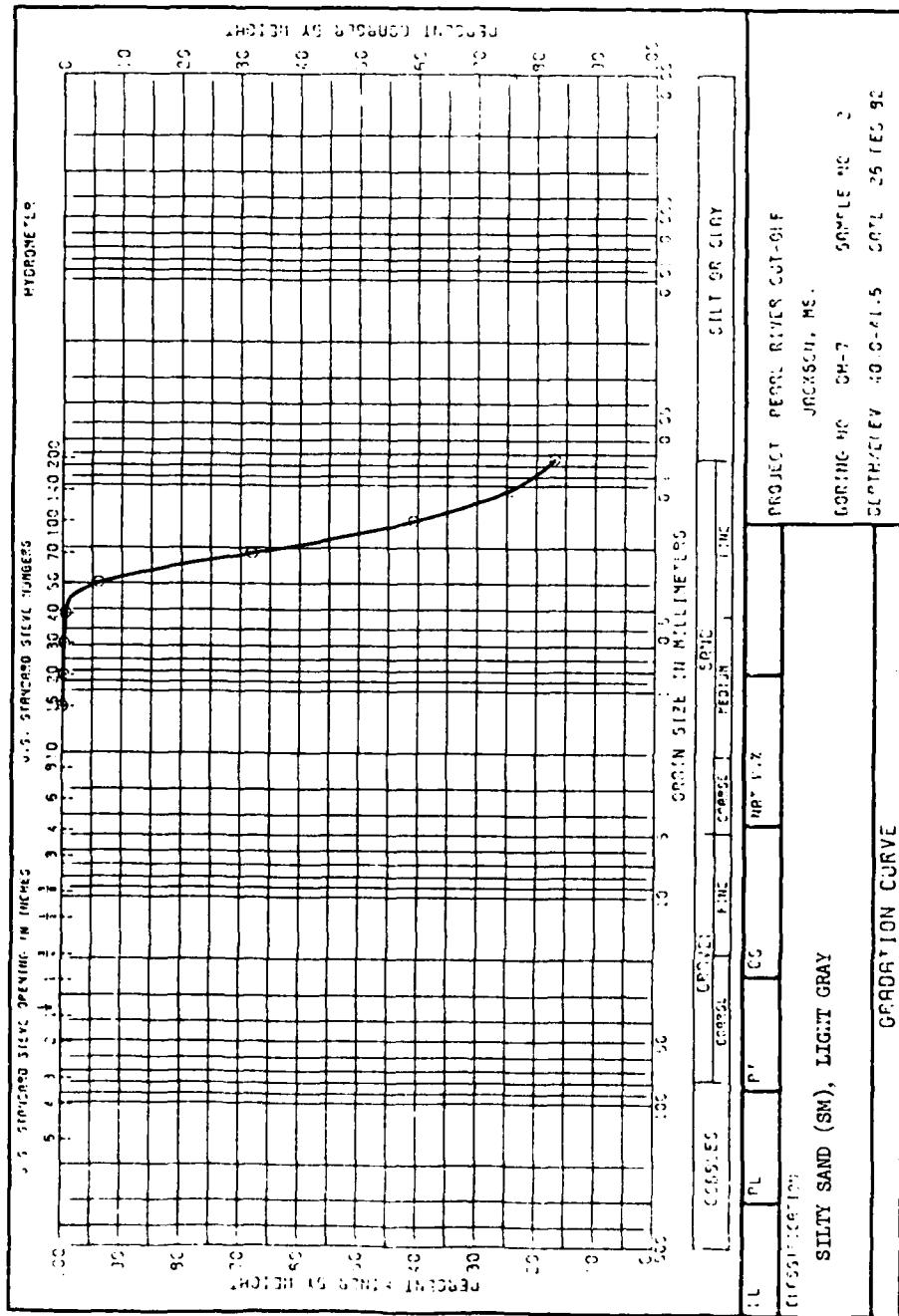


Figure 25. Aggregate grading curve for boring DH-7, depth 40.0 - 41.5

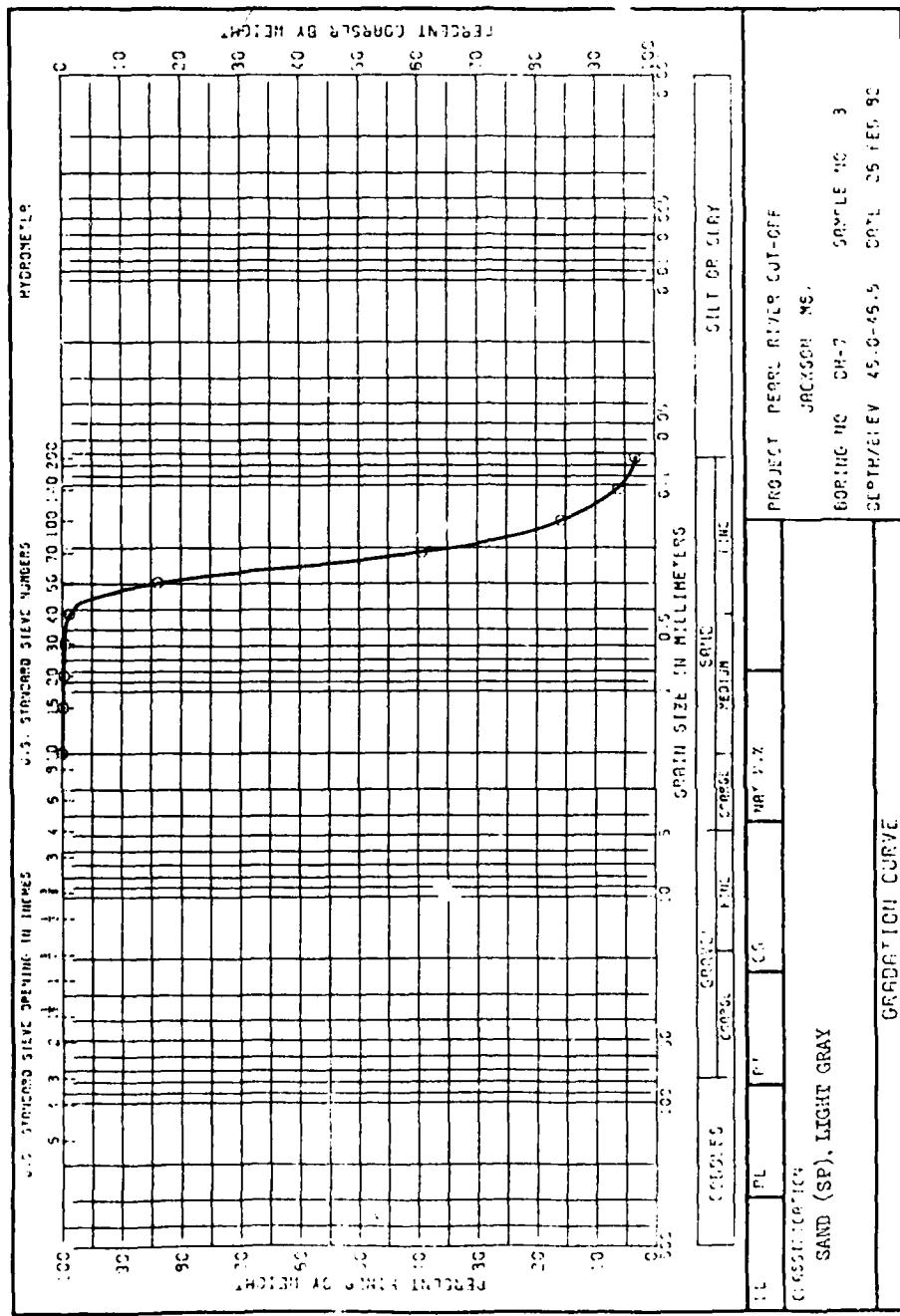


Figure 26. Aggregate grading curve for boring DH-7, depth 45.0 - 46.5

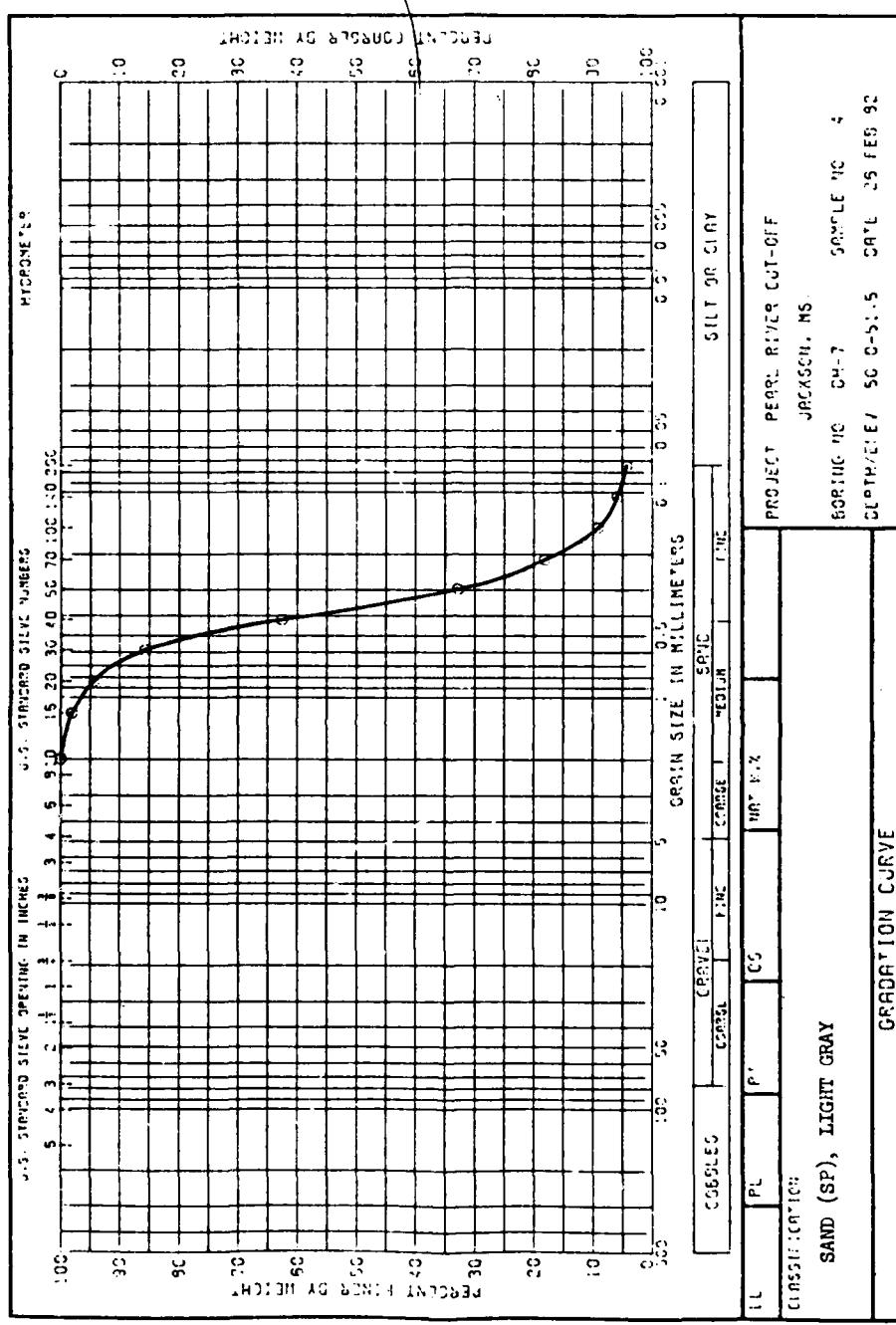


Figure 27. Aggregate grading curve for boring DH-7, depth 50.0 - 51.5

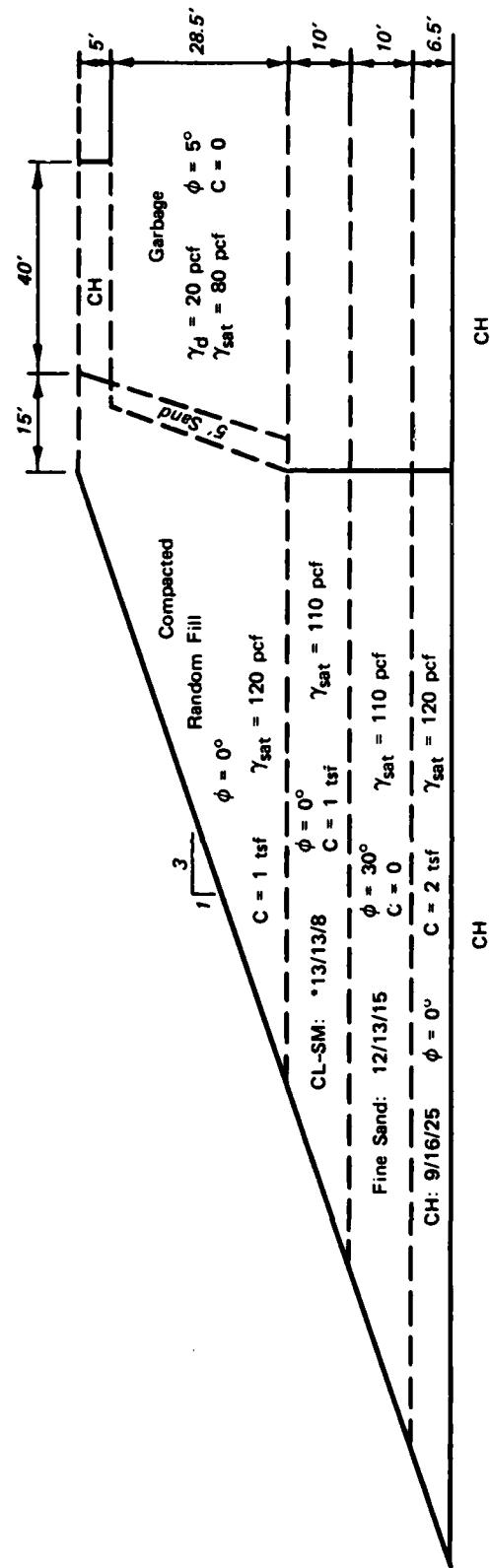
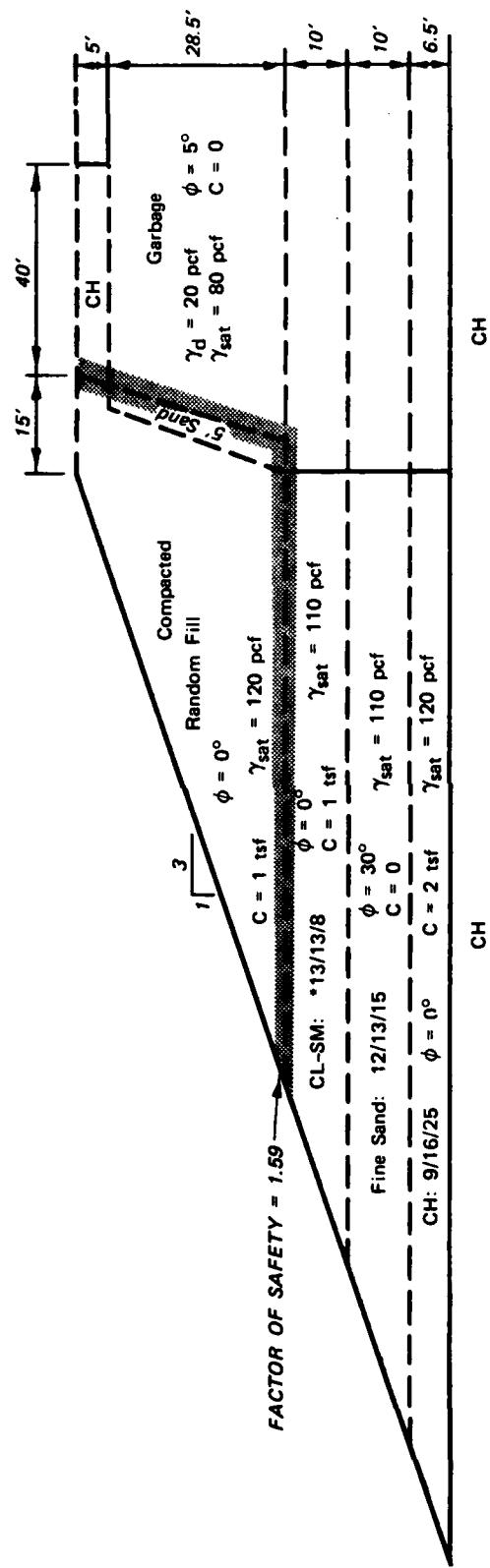
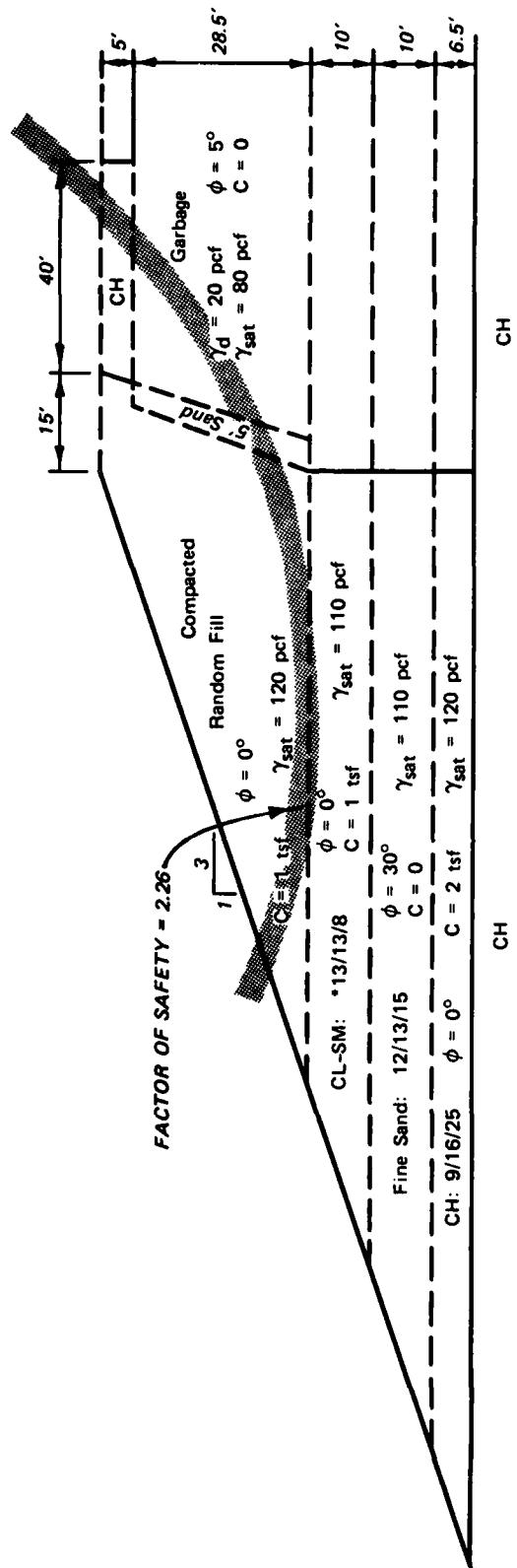


Figure 28. Cross-section configuration and soil parameters selected for stability analysis



*STANDARD PENETRATION TEST - 18-IN. DRIVE

Figure 29. Stability analysis-wedge method-failure plane along bottom surface of garbage



*STANDARD PENETRATION TEST - 18-IN. DRIVE

Figure 30. Stability analysis-wedge method-failure plane along bottom surface of garbage

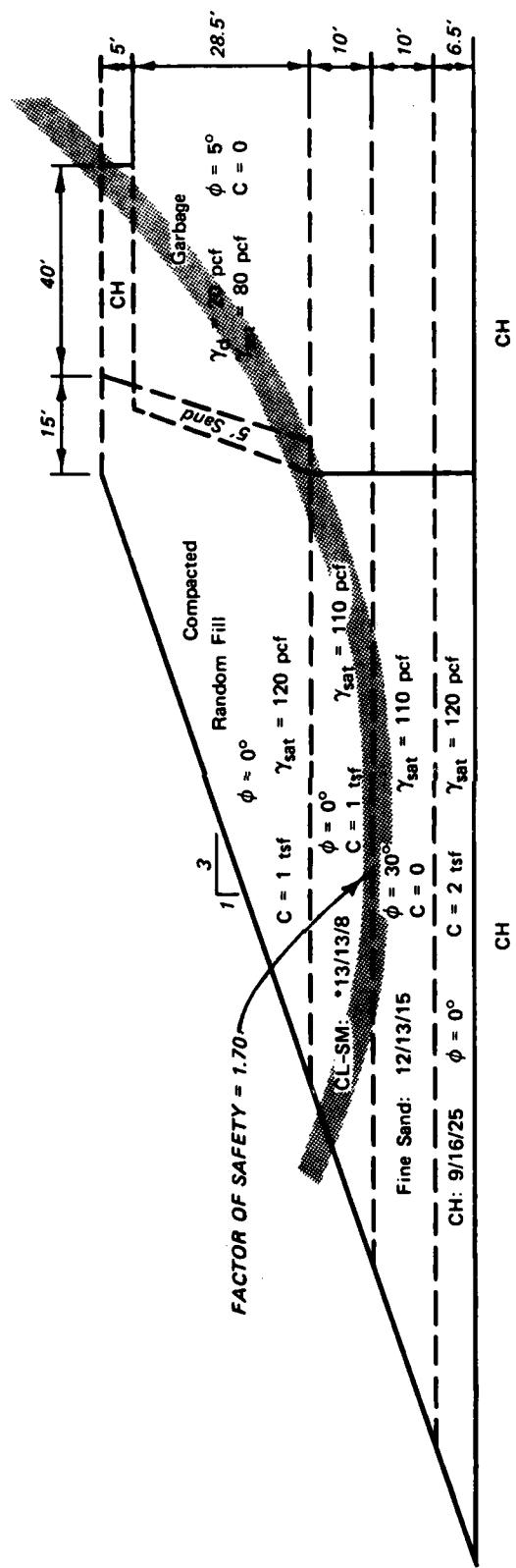
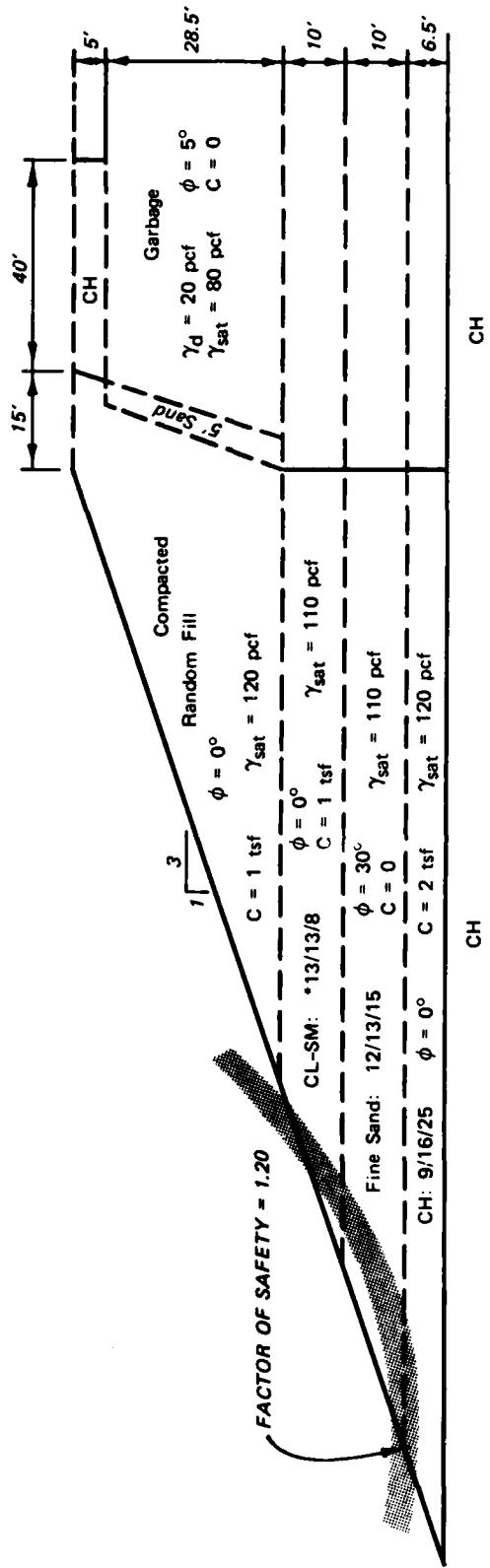


Figure 31. Stability analysis-arc method-tangent elevation at bottom surface of garbage



*STANDARD PENETRATION TEST - 18-IN. DRIVE
 Figure 32. Stability analysis-arc method-tangent elevation at bottom surface of CL-SM layer

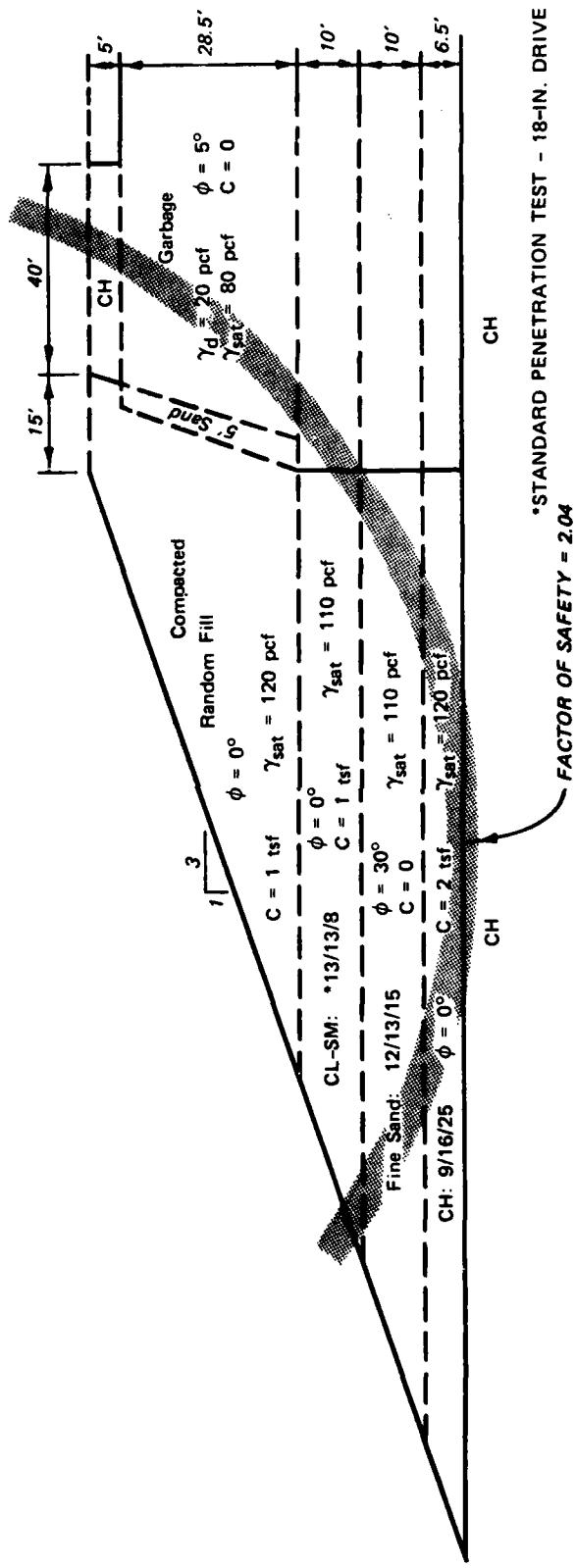


Figure 33. Stability analysis-arc method-tangent elevation at invert

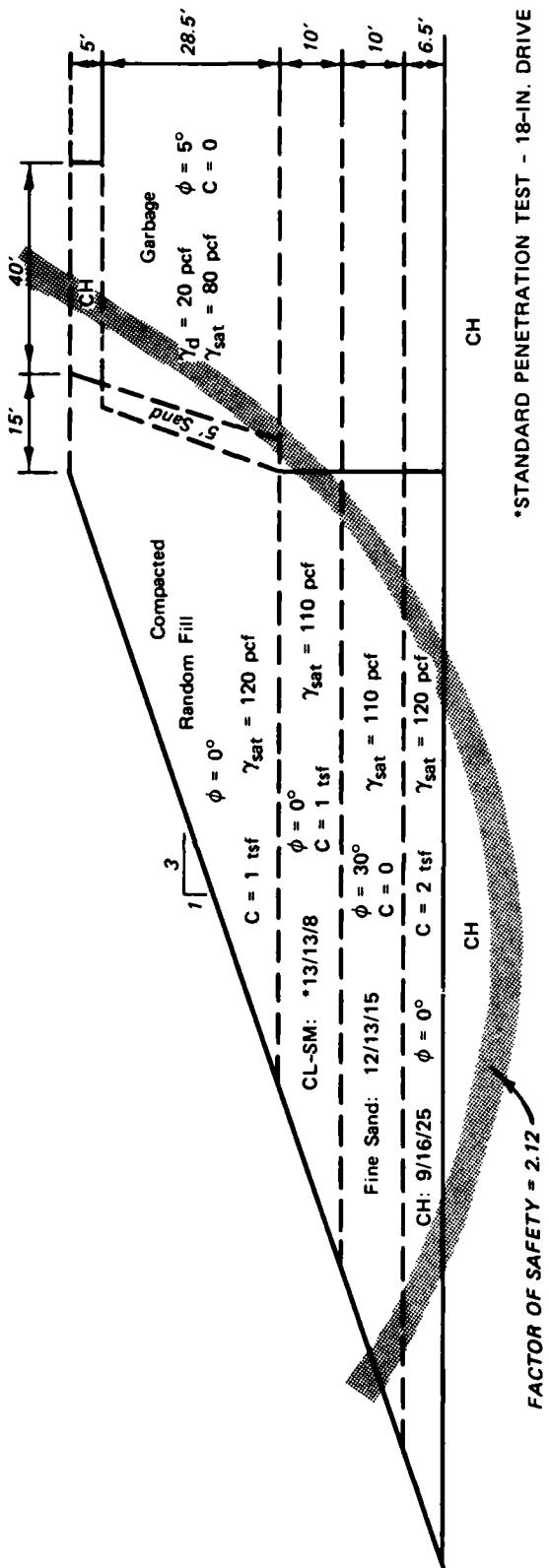
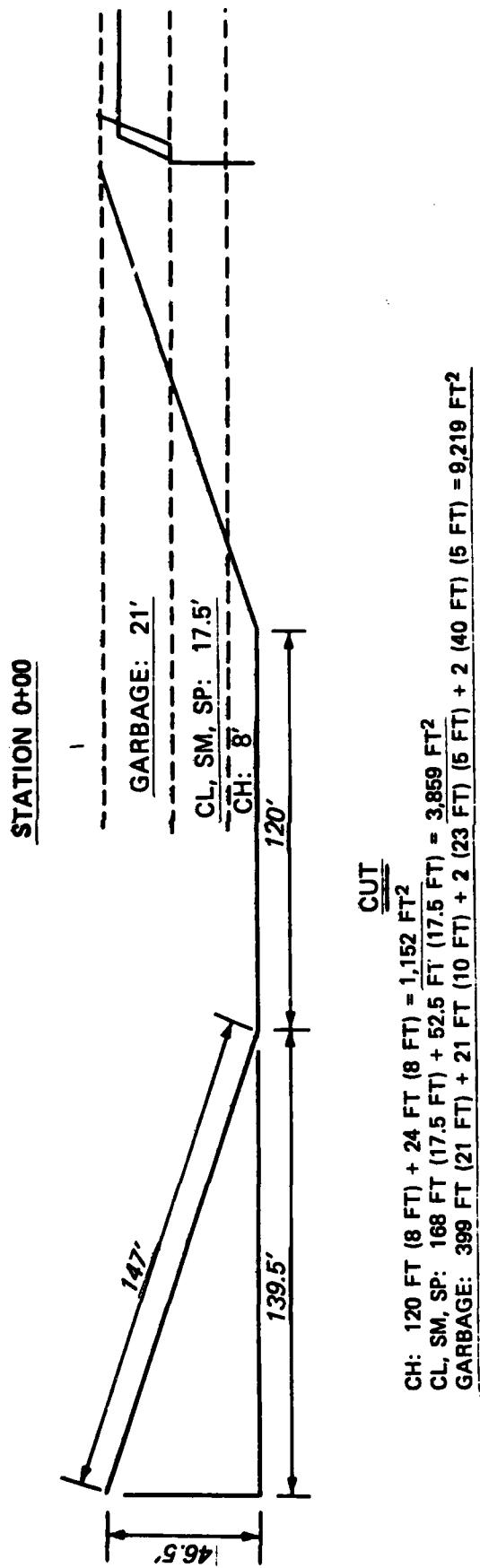
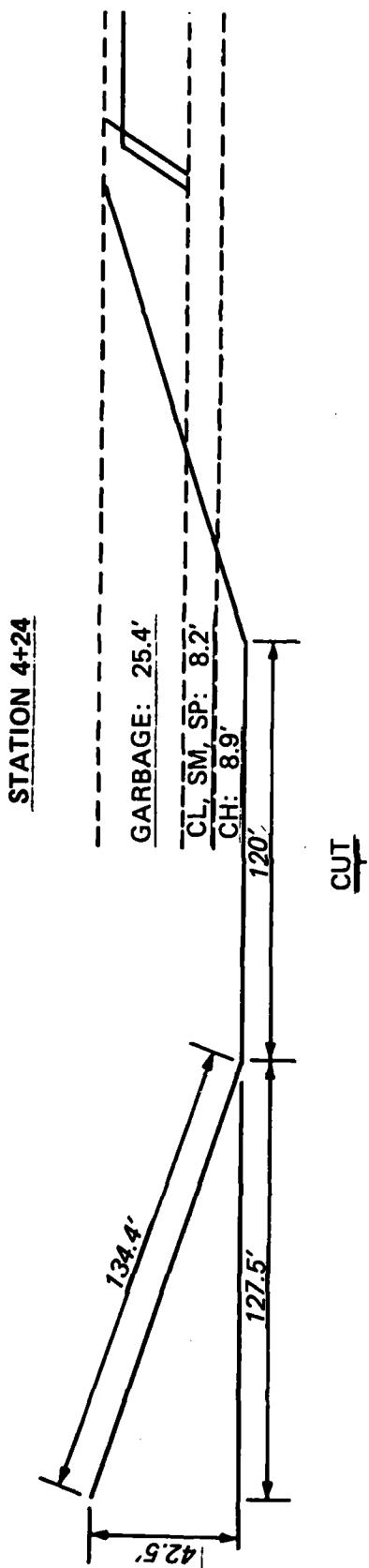


Figure 34. Stability analysis-arc method-tangent elevation 10 ft below invert



$$\begin{aligned}
 \text{CH: } & 120 \text{ FT (8 FT)} + 24 \text{ FT (8 FT)} = 1,152 \text{ FT}^2 \\
 \text{CL, SM, SP: } & 168 \text{ FT (17.5 FT)} + 52.5 \text{ FT (17.5 FT)} = 3,859 \text{ FT}^2 \\
 \text{GARBAGE: } & 399 \text{ FT (21 FT)} + 21 \text{ FT (10 FT)} + 2 (23 \text{ FT}) (5 \text{ FT}) = 9,219 \text{ FT}^2
 \end{aligned}$$

Figure 35. Cross section and end areas for earthwork computations - station 0+00



$CH: 120 \text{ FT (8.9 FT)} + 26.7 \text{ FT (8.9 FT)} = 1,306 \text{ FT}^2$
 $CL, SM, SP: 173.4 \text{ FT (8.2 FT)} + 24.6 \text{ FT (8.2 FT)} = 1,624 \text{ FT}^2$
GARBAGE: $375 \text{ FT (25.4 FT)} + 10 \text{ FT (25.4 FT)} + 30 \text{ FT (5 FT)} (2 \text{ FT}) + 2 \text{ FT (40 FT)} (5 \text{ FT}) = 10,479 \text{ FT}^2$

Figure 36. Cross section and end areas for earthwork computations - station 4+24

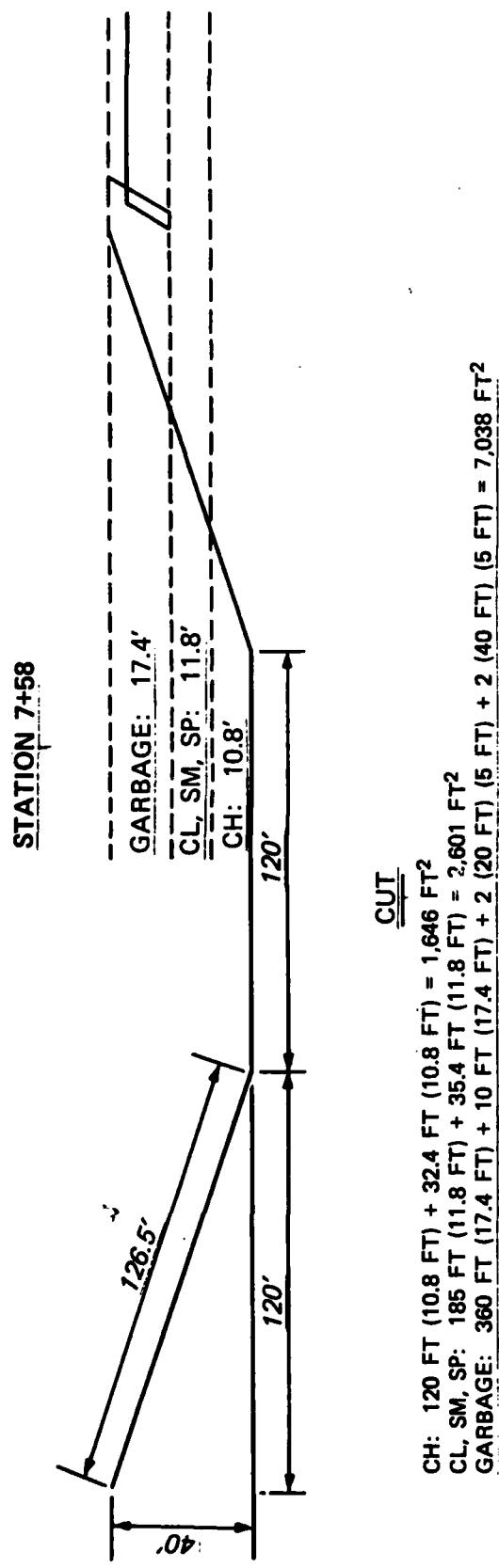
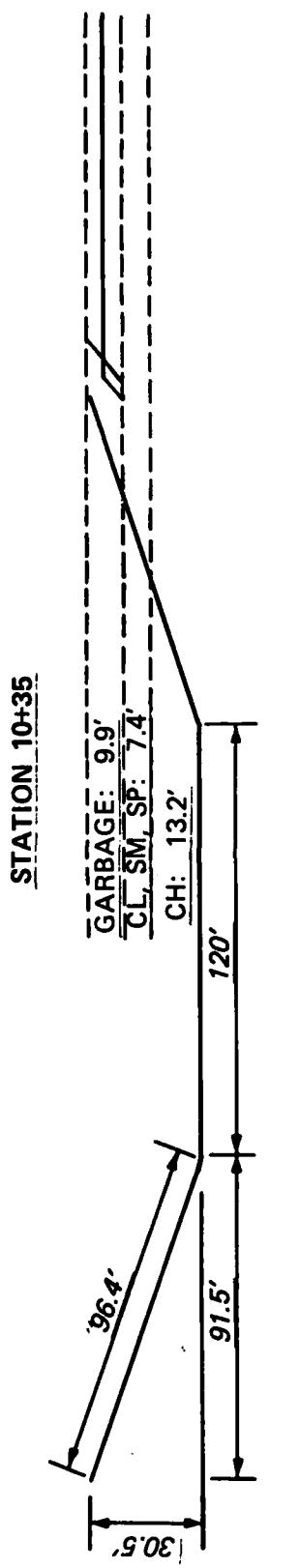


Figure 37. Cross section and end areas for earthwork computations - station 7+58



$CH: 120 \text{ FT} (13.2 \text{ FT}) + 39.6 \text{ FT} (13.2 \text{ FT}) = 2,107 \text{ FT}^2$
 $CL, SM, SP: 199.3 \text{ FT} (7.4 \text{ FT}) + 22.2 \text{ FT} (7.4 \text{ FT}) = 1,639 \text{ FT}^2$
 $GARBAGE: 303 \text{ FT} (9.9 \text{ FT}) + 10 \text{ FT} (9.9 \text{ FT}) + 2 (15 \text{ FT}) (5 \text{ FT}) + 2 (40 \text{ FT}) (5 \text{ FT}) = 3,649 \text{ FT}^2$

Figure 38. Cross section and end areas for earthwork computations - station 10+35

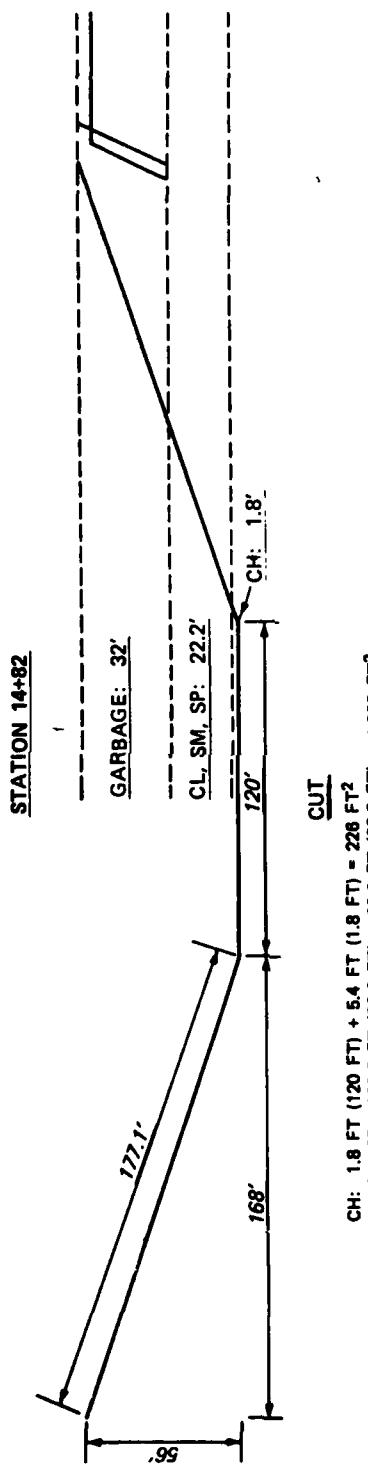
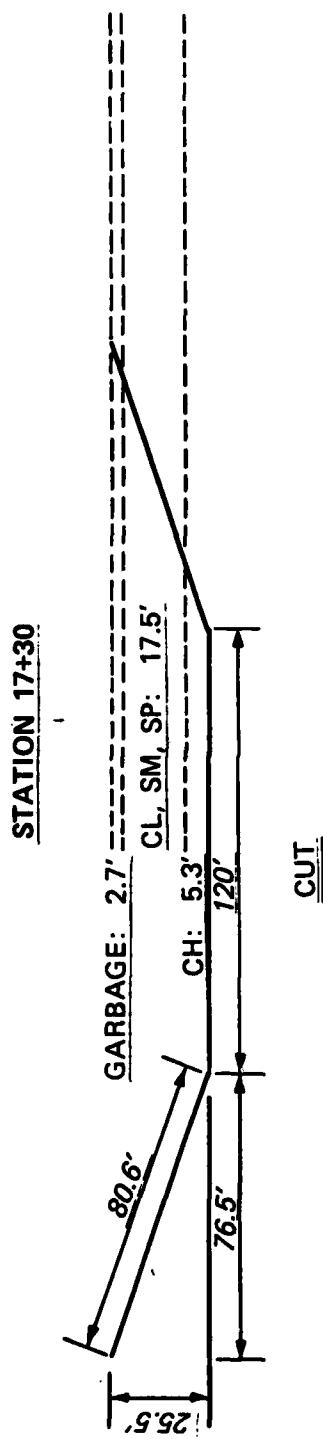


Figure 39. Cross section and end areas for earthwork computations - station 14+82



CH: 120 FT (5.3 FT) + 15.9 FT (5.3 FT) = 720 FT²
CL, SM, SP: 151.9 FT (17.5 FT) + 52.5 FT (17.5 FT) = 3,577 FT²
GARBAGE: 273 FT (27 FT) + 10 FT (27 FT) + 2 (10 FT) (5 FT) + 2 (40 FT) (5 FT) = 4,913 FT²

Figure 40. Cross section and end areas for earthwork computations - station 17+30

APPENDIX A

**SELECTED SAMPLE BORING LOGS FROM THE
MISSISSIPPI HIGHWAY DEPARTMENT**

MISSISSIPPI STATE HIGHWAY DEPARTMENT
TESTING DIVISION
SOIL BORING LOG

Hole No.: 25-1036 (Co. No. - Site No. - Hole No.)
 Project No.: FTS-51-E(9)91 P41 County: Hinds District No.: 2
 Station No.: 70+00 Offset: 6-1/4 Prop. Owner: J. S. Madison
 Date Started: 10-21-62 Date Completed: 10-21-62 Sampler Type: SPLIT SP-221
 Surface Elev.: 262.4 Hole Depth: 50'5" Sampler Hammer: Weight: 140 Type: V-6
 Casing Length: Drilled by: Holley Drilling
 Remarks: ~~vegetation - clear & mild~~ ~~residential foundation activities~~
~~over part Nine Channel Canal~~
~~water level on 11-4-62 = 14'~~

Data Submitted By: J. W. Green

| Sample Number | Depth | Elev. | S.S. Blows | | | Log | Sample Description |
|---------------|-------------|---------|------------|----|----|-----|--|
| | | | 6" | 6" | 6" | | |
| | | | | | | | Brown, silty clay. |
| 401-1235 | -10' | 252.4 | 11 | 19 | 22 | | Light brown, fine to medium grain sand. |
| | water level | 11-4-62 | | | | | |
| 402-153 | -20' | 242.4 | 3 | 14 | 19 | | Brown, medium to coarse grain damp sand. |
| 402-253 | -30' | 232.4 | 6 | 13 | 22 | # | 6-8' 5" sand & gravel top streaked |
| | | | | | | | Brown, carbonaceous clay with alternating layers of gray, fine sand. |
| 402-353 | -40' | 222.4 | 11 | 15 | 23 | # | |
| 402-453 | -50' | 212.4 | 60 | X | X | | T.D. 50'5" |
| | -60' | | | | | | |
| | -70' | | | | | | |
| | -80' | | | | | | |

Sand

Lime or Chalk

Silt

Clay or Shale

Gravel

Marl

Mica

Organic Material

Glaconite

Fossiliferous

MISSISSIPPI STATE HIGHWAY DEPARTMENT
TESTING DIVISION
SOIL BORING LOG

For

Drill Unit No. E
Hole No. 25-244-8 (Co. No. - Site No. - Hole No.)
Project No. I-TG-55-5 (E) 91 County: Hinds Sheet 1 of 1
Station No. 13+85 Offset: 50' W. Prop. Owner:
Date Started: 6-19-64 Date Completed: 6-19-64 Sampler Type: SPLIT SPON
Surface Elev.: 256.3 Hole Depth: 4 1/2' Sampler Hammer: Weight: 140 Type: 1/2"
Casing Length: Drilled by: Bobby Weller
Remarks: weather - partly cloudy & hot. Pearl River channel change

Data Submitted By: J. W. Green

| Sample Number | Depth | Elev. | S.S. Blows | | | Log | Sample Description |
|-----------------|-------------|--------------|------------|----|----|-----|---|
| | | | 6" | 6" | 6" | | |
| <u>476-13.5</u> | | | | | | | <u>Brown, fine grain sand,</u> |
| | <u>-10'</u> | <u>246.3</u> | | | | | <u>Brown, silty, iron oxide stained clay,</u> |
| <u>476-2.5</u> | <u>-20'</u> | <u>236.3</u> | | | | | <u>Gray, medium grain, wet sand. Small amount of pebbles at base</u> |
| | <u>-30'</u> | <u>226.3</u> | | | | | <u>Top Cock F. 1d Fm</u> |
| <u>476-25.5</u> | <u>-40'</u> | <u>216.3</u> | | | | | <u>Brown, carbonaceous, silty clay with numerous lignite streaks.</u> |
| | <u>-50'</u> | | | | | | <u>T.D. 41 1/2'</u> |
| | <u>-60'</u> | | | | | | |
| | | | | | | | |

| | | | | | | | |
|---------------|--|---------------|--|------------------|--|---------------|--|
| Sand | | Clay or Shale | | Mica | | Glaucosite | |
| Lime or Chalk | | Gravel | | Organic Material | | Fossiliferous | |
| Silt | | Marl | | | | | |

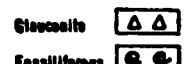
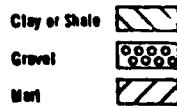
MISSISSIPPI STATE HIGHWAY DEPARTMENT
TESTING DIVISION
SOIL BORING LOG

For

Hole No. 10-205-7 (Co. No. - Site No. - Hole No.)
Project No. I-IG-55-2(B)91 Pt. II County: Rankin Sheet: 1 of 1
Station No. 25+00 Offset: 8 1/4 ft. N Prop. Owner: 5
Date Started: 11-13-63 Date Completed: 11-13-63 Sampler Type: Split spoon
Surface Elev.: 359.20 Hole Depth: 5 1/2' Sampler Hammer: Weight: 140 Type: Var
Casing Length: - Drilled by: Bobby Triplett
Remarks: weathered - clear cut, bridge foundation. Intersects
over existing Pearl River and survey
water level on 11-15-63 is 10'

Data Submitted By: J. W. Green

| Sample Number | Depth | Elev. | S.S. Blows | | | Log | Sample Description |
|------------------|-------------|-----------------|------------|----|----|-----|---|
| | | | 6" | 6" | 6" | | |
| | | | | | | | <u>Brown, clayey silt.</u> |
| | <u>-10'</u> | | | | | | <u>light brown, wet, medium grain sand.</u> |
| <u>106-63.5</u> | <u>-10'</u> | <u>13 13 17</u> | | | | | |
| | <u>-20'</u> | | | | | | <u>Top Muddy Branch Fm.</u> |
| <u>106-73.3</u> | <u>-20'</u> | <u>13 16 20</u> | | | | | <u>Blue-gray glauconitic, very limy, fossiliferous clay.</u> |
| | <u>-30'</u> | | | | | | <u>Green-gray, very glauconitic, very limy, fossiliferous Marl.</u> |
| <u>106-85</u> | <u>-30'</u> | <u>16 29 30</u> | | | | | |
| | <u>-40'</u> | | | | | | <u>Top Creekfield Fm.</u> |
| <u>106-93.5</u> | <u>-40'</u> | <u>10 20 37</u> | | | | | <u>Alternating thin layers of brown carbonaceous clay & brown, carbonaceous, slightly micaceous silt.</u> |
| | <u>-50'</u> | | | | | | |
| <u>106-103.5</u> | <u>-50'</u> | <u>28 39 46</u> | | | | | <u>T.D. 5 1/2'</u> |
| | <u>-60'</u> | | | | | | |
| | <u>-70'</u> | | | | | | |
| | <u>-80'</u> | | | | | | |



APPENDIX B

WELL BORING LOGS W-1 THROUGH W-6

Hole No. W-1

| DRILLING LOG | | DIVISION | INSTALLATION | | SHEET 1 of 1 SHEETS | | |
|--|-------|----------|--|--|------------------------|----------------------------|--|
| 1. PROJECT Pearl River Cut-off | | | 10. SIZE AND TYPE OF BIT Hollow Stem Auger | | | | |
| 2. LOCATION (Coordinates or Section) | | | 11. DATUM FOR ELEVATION SHOWN (TIDE or MSL) | | | | |
| 3. DRILLING AGENCY Waterways Experiment Station | | | 12. MANUFACTURER'S DESIGNATION OF DRILL | | | | |
| 4. HOLE NO. (As shown on drilling line) and No. marked | | W-1 | Failing | | | | |
| 5. NAME OF DRILLER Clyde Drake | | | 13. TOTAL NO. OF OVER-BURDEN SAMPLES TAKEN | | 0 | 0 | |
| 6. DIRECTION OF HOLE <input checked="" type="checkbox"/> VERTICAL <input type="checkbox"/> INCLINED --- DEG. FROM VERT. | | | 14. TOTAL NUMBER CORE BOXES | | 0 | | |
| 7. THICKNESS OF OVERTBURDEN 21.5 ft | | | 15. ELEVATION GROUND WATER 244.97' (7 Dec) | | | | |
| 8. DEPTH DRILLED INTO ROCK --- | | | 16. DATE HOLE STARTED 7 Dec 81 | | COMPLETED 8 Dec 81 | | |
| 9. TOTAL DEPTH OF HOLE 21.5 ft | | | 17. ELEVATION TOP OF HOLE 256.97 ft (MSL) | | | | |
| | | | 18. TOTAL CORE RECOVERY FOR BORING 0 | | | | |
| | | | 19. SIGNATURE OF INSPECTOR Charles Weller | | | | |
| ELEVATION | DEPTH | LEGEND | CLASSIFICATION OF MATERIALS (Description) | | 3. CORE RECOV- ERY | 4. BOX OR SAMPLE NO. | REMARKS (Drilling time, water loss, depth of weathering, etc., if significant) |
| 236.97 | | | clay-brown, micaceous with roots in upper few inches | | | | |
| 234.47 | | | silt-brown to gray, finely laminated | | | | |
| 232.97 | 5 | | silt-tan, clayey | | | | |
| | 10 | | | | | | |
| 244.97 | 15 | | sand-gray to white, fine to medium silty sand, micaceous; silt and fine sand content decrease with depth. | | | | |
| | 20 | | | | | | |
| 235.47 | | | Bottom of hole at 21.5 ft | | | | |

| DRILLING LOG | | | DIVISION | INSTALLATION | | | Hole No. W-2 | |
|--|-------|--------|--|--------------|--|------------------------|-------------------------|--|
| 1. PROJECT Pearl River Cut-off | | | Waterways Experiment Station | | | SHEET 1 OF 1 SHEETS | | |
| 2. LOCATION (Coordinates or Station) | | | 10. SIZE AND TYPE OF BIT Hollow stem auger | | | | | |
| 3. DRILLING AGENCY Waterways Experiment Station | | | 11. DATUM FOR ELEVATION SHOWN (TBM or MSL) MSL | | | | | |
| 4. HOLE NO. (As shown on drawing title and file number) | | | 12. MANUFACTURER'S DESIGNATION OF DRILL Failing | | | | | |
| 5. NAME OF DRILLER Clyde Drake | | | 13. TOTAL NO. OF OVER- BURDEN SAMPLES TAKEN | | | DISTURBED | UNDISTURBED | |
| 6. DIRECTION OF HOLE <input checked="" type="checkbox"/> VERTICAL <input type="checkbox"/> INCLINED --- DEG. FROM VERT. | | | 14. TOTAL NUMBER CORE BOXES --- | | | | | |
| 7. THICKNESS OF OVERBURDEN 22.0 ft | | | 15. ELEVATION GROUND WATER 259.98 ft (8 Dec) | | | | | |
| 8. DEPTH DRILLED INTO ROCK --- | | | 16. DATE HOLE STARTED 8 Dec 81 | | | COMPLETED | | |
| 9. TOTAL DEPTH OF HOLE 22.0 ft | | | 17. ELEVATION TOP OF HOLE 268.98 ft (MSL) | | | | | |
| | | | 18. TOTAL CORE RECOVERY FOR BORING 0 | | | | | |
| | | | 19. SIGNATURE OF INSPECTOR <i>Charles White</i> | | | | | |
| ELEVATION | DEPTH | LEGEND | CLASSIFICATION OF MATERIALS (Description) | | | CORE RECOV. % | BOX OR SAMPLE NO. | REMARKS (Drilling time, water loss, depth of weathering, etc., if significant) |
| 268.98 | | | clay - brown, silty with grass roots in upper 0.5 ft | | | | | A hollow stem auger with a wood knock out plug in the bottom was used to install the well. The sand was allowed to collapse around the well screen when the auger was pulled. |
| 266.98 | | | silt - brown to gray, finely laminated, clayey-silt; clay content decreases with depth | | | | | An open auger was used to drill to the water table, which was at the base of the silt (9. ft). |
| 259.48 | 5 | | sand - white, medium, mica- ceous | | | | | |
| | 10 | | | | | | | |
| | 15 | | | | | | | |
| | 20 | | | | | | | |
| 246.98 | | | Bottom of hole at 22.0 ft | | | | | |

Hole No. W-3

| DRILLING LOG | | | DIVISION | INSTALLATION | SHEET 1 OF 1 SHEETS | |
|--|-------|--------|--|---------------|------------------------|--|
| 1. PROJECT | | | Waterways Experiment Station | | | |
| Pearl River Cut-off | | | 10. SIZE AND TYPE OF BIT Hollow stem auger | | | |
| 2. LOCATION (Coordinates of Station) 17°35' 41 ft E of S | | | 11. DATUM FOR ELEVATION SHOWN (TBM or MSL) | | | |
| 3. DRILLING AGENCY Waterways Experiment Station | | | 12. MANUFACTURER'S DESIGNATION OF DRILL | | | |
| 4. HOLE NO. (As shown on drawing title and file number) W-3 | | | MSL | | | |
| 5. NAME OF DRILLER Clyde Drake | | | 13. TOTAL NO. OF OVER- BURDEN SAMPLES TAKEN | | | |
| 6. DIRECTION OF HOLE <input checked="" type="checkbox"/> VERTICAL <input type="checkbox"/> INCLINED --- DEG. FROM VERT. | | | 14. TOTAL NUMBER CORE BOXES --- | | | |
| 7. THICKNESS OF OVERTBURDEN 31.0 ft | | | 15. ELEVATION GROUND WATER 240.80 ft (11 Dec) | | | |
| 8. DEPTH DRILLED INTO ROCK --- | | | 16. DATE, HOLE STARTED 11 Dec 81 COMPLETED 12 Dec 81 | | | |
| 9. TOTAL DEPTH OF HOLE 31.0 ft | | | 17. ELEVATION TOP OF HOLE 254.80 ft (MSL) | | | |
| | | | 18. TOTAL CORE RECOVERY FOR BORING --- | | | |
| | | | 19. SIGNATURE OF INSPECTOR Charlie Whalen | | | |
| ELEVATION | DEPTH | LEGEND | CLASSIFICATION OF MATERIALS (Description) | CORE RECOVERY | BOX OR SAMPLE NO. | REMARKS (Drilling time, water loss, depth of soaking, etc., if significant) |
| 254.80 | 5 | | sandy - silt, brown (pushed over site by bulldozer) | | | Drilled with hollow stem auger, pulled auger and used 6" steel casing to place gravel pack around well screen. |
| 252.80 | 5 | | silt - tan to brown, finely laminated with some fine sand; increase in size and quantity of sand with depth | | | |
| 239.30 | 10 | | | | | |
| 239.30 | 15 | | sand - gray, fine, silty sand with scattered silt lenses; wood fragments up to 2" long scattered throughout; pea gravel a few inches thick at base | | | An open auger was used to drill to the water table (15.4 ft). |
| 232.30 | 20 | | | | | |
| 232.30 | 25 | | clay - green, fossiliferous; stiff, plastic (Yazoo clay) | | | |
| 227.42 | | | Bottom of hole at 27.38 ft | | | |

Hole No. W-4

| DRILLING LOG | | DIVISION | | INSTALLATION | | SHEET 1 of 1 SHEETS | |
|---|--|--|--|--|--|--|--|
| 1. PROJECT Pearl River Cut-off | | 2. LOCATION (Coordinates or Station) 14 + 42 on 6 | | 3. DRILLING AGENCY Waterways Experiment Station | | 4. SIZE AND TYPE OF BIT Hollow stem auger | |
| 5. DRILLING AGENCY Clyde Drake | | 6. Datum for Elevation Shown (TBM or MSL) MSL | | 7. MANUFACTURER'S DESIGNATION OF DRILL Falling | | 8. TOTAL NO. OF OVER-BURDEN SAMPLES TAKEN --- | |
| 9. MOLE NO. (As shown on drilling title and file number) W-4 | | 10. TOTAL NUMBER CORE BOXES --- | | 11. ELEVATION GROUND WATER 248.23 ft (15 Dec) | | 12. TOTAL ELEVATION FOR BORING 281.63 ft (MSL) | |
| 13. DIRECTION OF HOLE <input checked="" type="checkbox"/> VERTICAL <input type="checkbox"/> INCLINED --- DEG. FROM VERT. | | 14. DATE HOLE 14 Dec 81 | | 15. TOTAL CORE RECOVERY FOR BORING 0 | | 16. SIGNATURE OF INSPECTOR Charles White | |
| 17. THICKNESS OF OVERTBURDEN 44.0 ft | | 18. REMARKS (Drilling time, water loss, depth of weathering, etc., if significant) | | 19. TOTAL CORE RECOVERY FOR BORING 0 | | 20. REMARKS (Drilling time, water loss, depth of weathering, etc., if significant) | |
| 21. DEPTH DRILLED INTO ROCK --- | | 22. ELEVATION 281.63 | | 23. DEPTH 5 | | 24. ELEVATION 10 | |
| 25. TOTAL DEPTH OF HOLE 44.0 ft | | 26. DEPTH 15 | | 27. ELEVATION 15 | | 28. DEPTH 20 | |
| 29. ELEVATION 25 | | 30. DEPTH 25 | | 31. ELEVATION 25 | | 32. DEPTH 30 | |
| 33. ELEVATION 255.63 | | 34. DEPTH 255.63 | | 35. ELEVATION 254.63 | | 36. DEPTH 30 | |
| 37. ELEVATION 237.63 | | 38. DEPTH 237.63 | | 39. ELEVATION 237.63 | | 40. DEPTH 35 | |
| 41. ELEVATION 237.63 | | 42. DEPTH 237.63 | | 43. ELEVATION 237.63 | | 44. DEPTH 40 | |
| 45. ELEVATION 237.63 | | 46. DEPTH 237.63 | | 47. ELEVATION 237.63 | | 48. DEPTH 45 | |
| 49. ELEVATION 237.63 | | 50. DEPTH 237.63 | | 51. ELEVATION 237.63 | | 52. DEPTH 50 | |
| 53. ELEVATION 237.63 | | 54. DEPTH 237.63 | | 55. ELEVATION 237.63 | | 56. DEPTH 55 | |
| 57. ELEVATION 237.63 | | 58. DEPTH 237.63 | | 59. ELEVATION 237.63 | | 60. DEPTH 60 | |
| 61. ELEVATION 237.63 | | 62. DEPTH 237.63 | | 63. ELEVATION 237.63 | | 64. DEPTH 65 | |
| 65. ELEVATION 237.63 | | 66. DEPTH 237.63 | | 67. ELEVATION 237.63 | | 68. DEPTH 65 | |
| 69. ELEVATION 237.63 | | 70. DEPTH 237.63 | | 71. ELEVATION 237.63 | | 72. DEPTH 70 | |
| 73. ELEVATION 237.63 | | 74. DEPTH 237.63 | | 75. ELEVATION 237.63 | | 76. DEPTH 75 | |
| 77. ELEVATION 237.63 | | 78. DEPTH 237.63 | | 79. ELEVATION 237.63 | | 80. DEPTH 77 | |
| 81. ELEVATION 237.63 | | 82. DEPTH 237.63 | | 83. ELEVATION 237.63 | | 84. DEPTH 81 | |
| 85. ELEVATION 237.63 | | 86. DEPTH 237.63 | | 87. ELEVATION 237.63 | | 88. DEPTH 85 | |
| 89. ELEVATION 237.63 | | 90. DEPTH 237.63 | | 91. ELEVATION 237.63 | | 92. DEPTH 89 | |
| 93. ELEVATION 237.63 | | 94. DEPTH 237.63 | | 95. ELEVATION 237.63 | | 96. DEPTH 93 | |
| 97. ELEVATION 237.63 | | 98. DEPTH 237.63 | | 99. ELEVATION 237.63 | | 100. DEPTH 97 | |
| 101. ELEVATION 237.63 | | 102. DEPTH 237.63 | | 103. ELEVATION 237.63 | | 104. DEPTH 101 | |
| 105. ELEVATION 237.63 | | 106. DEPTH 237.63 | | 107. ELEVATION 237.63 | | 108. DEPTH 105 | |
| 109. ELEVATION 237.63 | | 110. DEPTH 237.63 | | 111. ELEVATION 237.63 | | 112. DEPTH 109 | |
| 113. ELEVATION 237.63 | | 114. DEPTH 237.63 | | 115. ELEVATION 237.63 | | 116. DEPTH 113 | |
| 117. ELEVATION 237.63 | | 118. DEPTH 237.63 | | 119. ELEVATION 237.63 | | 120. DEPTH 117 | |
| 121. ELEVATION 237.63 | | 122. DEPTH 237.63 | | 123. ELEVATION 237.63 | | 124. DEPTH 121 | |
| 125. ELEVATION 237.63 | | 126. DEPTH 237.63 | | 127. ELEVATION 237.63 | | 128. DEPTH 125 | |
| 129. ELEVATION 237.63 | | 130. DEPTH 237.63 | | 131. ELEVATION 237.63 | | 132. DEPTH 129 | |
| 133. ELEVATION 237.63 | | 134. DEPTH 237.63 | | 135. ELEVATION 237.63 | | 136. DEPTH 133 | |
| 137. ELEVATION 237.63 | | 138. DEPTH 237.63 | | 139. ELEVATION 237.63 | | 140. DEPTH 137 | |
| 141. ELEVATION 237.63 | | 142. DEPTH 237.63 | | 143. ELEVATION 237.63 | | 144. DEPTH 141 | |
| 145. ELEVATION 237.63 | | 146. DEPTH 237.63 | | 147. ELEVATION 237.63 | | 148. DEPTH 145 | |
| 149. ELEVATION 237.63 | | 150. DEPTH 237.63 | | 151. ELEVATION 237.63 | | 152. DEPTH 149 | |
| 153. ELEVATION 237.63 | | 154. DEPTH 237.63 | | 155. ELEVATION 237.63 | | 156. DEPTH 153 | |
| 157. ELEVATION 237.63 | | 158. DEPTH 237.63 | | 159. ELEVATION 237.63 | | 160. DEPTH 157 | |
| 161. ELEVATION 237.63 | | 162. DEPTH 237.63 | | 163. ELEVATION 237.63 | | 164. DEPTH 161 | |
| 165. ELEVATION 237.63 | | 166. DEPTH 237.63 | | 167. ELEVATION 237.63 | | 168. DEPTH 165 | |
| 169. ELEVATION 237.63 | | 170. DEPTH 237.63 | | 171. ELEVATION 237.63 | | 172. DEPTH 169 | |
| 173. ELEVATION 237.63 | | 174. DEPTH 237.63 | | 175. ELEVATION 237.63 | | 176. DEPTH 173 | |
| 177. ELEVATION 237.63 | | 178. DEPTH 237.63 | | 179. ELEVATION 237.63 | | 180. DEPTH 177 | |
| 181. ELEVATION 237.63 | | 182. DEPTH 237.63 | | 183. ELEVATION 237.63 | | 184. DEPTH 181 | |
| 185. ELEVATION 237.63 | | 186. DEPTH 237.63 | | 187. ELEVATION 237.63 | | 188. DEPTH 185 | |
| 189. ELEVATION 237.63 | | 190. DEPTH 237.63 | | 191. ELEVATION 237.63 | | 192. DEPTH 189 | |
| 193. ELEVATION 237.63 | | 194. DEPTH 237.63 | | 195. ELEVATION 237.63 | | 196. DEPTH 193 | |
| 197. ELEVATION 237.63 | | 198. DEPTH 237.63 | | 199. ELEVATION 237.63 | | 200. DEPTH 197 | |
| 201. ELEVATION 237.63 | | 202. DEPTH 237.63 | | 203. ELEVATION 237.63 | | 204. DEPTH 201 | |
| 205. ELEVATION 237.63 | | 206. DEPTH 237.63 | | 207. ELEVATION 237.63 | | 208. DEPTH 205 | |
| 209. ELEVATION 237.63 | | 210. DEPTH 237.63 | | 211. ELEVATION 237.63 | | 212. DEPTH 209 | |
| 213. ELEVATION 237.63 | | 214. DEPTH 237.63 | | 215. ELEVATION 237.63 | | 216. DEPTH 213 | |
| 217. ELEVATION 237.63 | | 218. DEPTH 237.63 | | 219. ELEVATION 237.63 | | 220. DEPTH 217 | |
| 221. ELEVATION 237.63 | | 222. DEPTH 237.63 | | 223. ELEVATION 237.63 | | 224. DEPTH 221 | |
| 225. ELEVATION 237.63 | | 226. DEPTH 237.63 | | 227. ELEVATION 237.63 | | 228. DEPTH 225 | |
| 229. ELEVATION 237.63 | | 230. DEPTH 237.63 | | 231. ELEVATION 237.63 | | 232. DEPTH 229 | |
| 233. ELEVATION 237.63 | | 234. DEPTH 237.63 | | 235. ELEVATION 237.63 | | 236. DEPTH 233 | |
| 237. ELEVATION 237.63 | | 238. DEPTH 237.63 | | 239. ELEVATION 237.63 | | 240. DEPTH 237 | |
| 241. ELEVATION 237.63 | | 242. DEPTH 237.63 | | 243. ELEVATION 237.63 | | 244. DEPTH 241 | |
| 245. ELEVATION 237.63 | | 246. DEPTH 237.63 | | 247. ELEVATION 237.63 | | 248. DEPTH 245 | |
| 249. ELEVATION 237.63 | | 250. DEPTH 237.63 | | 251. ELEVATION 237.63 | | 252. DEPTH 249 | |
| 253. ELEVATION 237.63 | | 254. DEPTH 237.63 | | 255. ELEVATION 237.63 | | 256. DEPTH 253 | |
| 257. ELEVATION 237.63 | | 258. DEPTH 237.63 | | 259. ELEVATION 237.63 | | 260. DEPTH 257 | |
| 261. ELEVATION 237.63 | | 262. DEPTH 237.63 | | 263. ELEVATION 237.63 | | 264. DEPTH 261 | |
| 265. ELEVATION 237.63 | | 266. DEPTH 237.63 | | 267. ELEVATION 237.63 | | 268. DEPTH 265 | |
| 269. ELEVATION 237.63 | | 270. DEPTH 237.63 | | 271. ELEVATION 237.63 | | 272. DEPTH 269 | |
| 273. ELEVATION 237.63 | | 274. DEPTH 237.63 | | 275. ELEVATION 237.63 | | 276. DEPTH 273 | |
| 277. ELEVATION 237.63 | | 278. DEPTH 237.63 | | 279. ELEVATION 237.63 | | 280. DEPTH 277 | |
| 281. ELEVATION 237.63 | | 282. DEPTH 237.63 | | 283. ELEVATION 237.63 | | 284. DEPTH 281 | |
| 285. ELEVATION 237.63 | | 286. DEPTH 237.63 | | 287. ELEVATION 237.63 | | 288. DEPTH 285 | |
| 289. ELEVATION 237.63 | | 290. DEPTH 237.63 | | 291. ELEVATION 237.63 | | 292. DEPTH 289 | |
| 293. ELEVATION 237.63 | | 294. DEPTH 237.63 | | 295. ELEVATION 237.63 | | 296. DEPTH 293 | |
| 297. ELEVATION 237.63 | | 298. DEPTH 237.63 | | 299. ELEVATION 237.63 | | 300. DEPTH 297 | |
| 301. ELEVATION 237.63 | | 302. DEPTH 237.63 | | 303. ELEVATION 237.63 | | 304. DEPTH 301 | |
| 305. ELEVATION 237.63 | | 306. DEPTH 237.63 | | 307. ELEVATION 237.63 | | 308. DEPTH 305 | |
| 309. ELEVATION 237.63 | | 310. DEPTH 237.63 | | 311. ELEVATION 237.63 | | 312. DEPTH 309 | |
| 313. ELEVATION 237.63 | | 314. DEPTH 237.63 | | 315. ELEVATION 237.63 | | 316. DEPTH 313 | |
| 317. ELEVATION 237.63 | | 318. DEPTH 237.63 | | 319. ELEVATION 237.63 | | 320. DEPTH 317 | |
| 321. ELEVATION 237.63 | | 322. DEPTH 237.63 | | 323. ELEVATION 237.63 | | 324. DEPTH 321 | |
| 325. ELEVATION 237.63 | | 326. DEPTH 237.63 | | 327. ELEVATION 237.63 | | 328. DEPTH 325 | |
| 329. ELEVATION 237.63 | | 330. DEPTH 237.63 | | 331. ELEVATION 237.63 | | 332. DEPTH 329 | |
| 333. ELEVATION 237.63 | | 334. DEPTH 237.63 | | 335. ELEVATION 237.63 | | 336. DEPTH 333 | |
| 337. ELEVATION 237.63 | | 338. DEPTH 237.63 | | 339. ELEVATION 237.63 | | 340. DEPTH 337 | |
| 341. ELEVATION 237.63 | | 342. DEPTH 237.63 | | 343. ELEVATION 237.63 | | 344. DEPTH 341 | |
| 345. ELEVATION 237.63 | | 346. DEPTH 237.63 | | 347. ELEVATION 237.63 | | 348. DEPTH 345 | |
| 349. ELEVATION 237.63 | | 350. DEPTH 237.63 | | 351. ELEVATION 237.63 | | 352. DEPTH 349 | |
| 353. ELEVATION 237.63 | | 354. DEPTH 237.63 | | 355. ELEVATION 237.63 | | 356. DEPTH 353 | |
| 357. ELEVATION 237.63 | | 358. DEPTH 237.63 | | 359. ELEVATION 237.63 | | 360. DEPTH 357 | |
| 361. ELEVATION 237.63 | | 362. DEPTH 237.63 | | 363. ELEVATION 237.63 | | 364. DEPTH 361 | |
| 365. ELEVATION 237.63 | | 366. DEPTH 237.63 | | 367. ELEVATION 237.63 | | 368. DEPTH 365 | |
| 369. ELEVATION 237.63 | | 370. DEPTH 237.63 | | 371. ELEVATION 237.63 | | 372. DEPTH 369 | |
| 373. ELEVATION 237.63 | | 374. DEPTH 237.63 | | 375. ELEVATION 237.63 | | 376. DEPTH 373 | |
| 377. ELEVATION 237.63 | | 378. DEPTH 237.63 | | 379. ELEVATION 237.63 | | 380. DEPTH 377 | |
| 381. ELEVATION 237.63 | | 382. DEPTH 237.63 | | 383. ELEVATION 237.63 | | 384. DEPTH 381 | |
| 385. ELEVATION 237.63 | | 386. DEPTH 237.63 | | 387. ELEVATION 237.63 | | 388. DEPTH 385 | |
| 389. ELEVATION 237.63 | | 390. DEPTH 237.63 | | 391. ELEVATION 237.63 | | 392. DEPTH 389 | |
| 393. ELEVATION 237.63 | | 394. DEPTH 237.63 | | 395. ELEVATION 237.63 | | 396. DEPTH 393 | |
| 397. ELEVATION 237.63 | | 398. DEPTH 237.63 | | 399. ELEVATION 237.63 | | 400. DEPTH 397 | |
| 401. ELEVATION 237.63 | | 402. DEPTH 237.63 | | 403. ELEVATION 237.63 | | 404. DEPTH 401 | |
| 405. ELEVATION 237.63 | | 406. DEPTH 237.63 | | 407. ELEVATION 237.63 | | 408. DEPTH 405 | |
| 409. ELEVATION 237.63 | | 410. DEPTH 237.63 | | 411. ELEVATION 237.63 | | 412. DEPTH 409 | |
| 413. ELEVATION 237.63 | | 414. DEPTH 237.63 | | 415. ELEVATION 237.63 | | 416. DEPTH 413 | |
| 417. ELEVATION 237.63 | | 418. DEPTH 237.63 | | 419. ELEVATION 237.63 | | 420. DEPTH 417 | |
| 421. ELEVATION 237.63 | | 422. DEPTH 237.63 | | 423. ELEVATION 237.63 | | 424. DEPTH 421 | |
| 425. ELEVATION 237.63 | | 426. DEPTH 237.63 | | 427. ELEVATION 237.63 | | 428. DEPTH 425 | |
| 429. ELEVATION 237.63 | | 430. DEPTH 237.63 | | 431. ELEVATION 237.63 | | 432. DEPTH 429 | |
| 433. ELEVATION 237.63 | | 434. DEPTH 237.63 | | 435. ELEVATION 237.63 | | 436. DEPTH 433 | |
| 437. ELEVATION 237.63 | | 438. DEPTH 237.63 | | 439. ELEVATION 237.63 | | 440. DEPTH 437 | |
| 441. ELEVATION 237.63 | | 442. DEPTH 237.63 | | 443. ELEVATION 237.63 | | 444. DEPTH 441 | |
| 445. ELEVATION 237.63 | | 446. DEPTH 237.63 | | 447. ELEVATION 237.63 | | 448. DEPTH 445 | |
| 449. ELEVATION 237.63 | | 450. DEPTH 237.63 | | 451. ELEVATION 237.63 | | 452. DEPTH 449 | |
| 453. ELEVATION 237.63 | | 454. DEPTH 237.63 | | 455. ELEVATION 237.63 | | 456. DEPTH 453 | |
| 457. ELEVATION 237.63 | | 458. DEPTH 237.63 | | 459. ELEVATION 237.63 | | 460. DEPTH 457 | |
| 461. ELEVATION 237.63 | | 462. DEPTH 237.63 | | 463. ELEVATION 237.63 | | 464. DEPTH 461 | |
| 465. ELEVATION 237.63 | | 466. DEPTH 237.63 | | 467. ELEVATION 237.63 | | 468. DEPTH 465 | |
| 469. ELEVATION 237.63 | | 470. DEPTH 237.63 | | 471. ELEVATION 237.63 | | 472. DEPTH 469 | |
| 473. ELEVATION 237.63 | | 474. DEPTH 237.63 | | 475. ELEVATION 237.63 | | 476. DEPTH 473 | |
| 477. ELEVATION 237.63 | | 478. DEPTH 237.63 | | 479. ELEVATION 237.63 | | 480. DEPTH 477 | |
| 481. ELEVATION 237.63 | | 482. DEPTH 237.63 | | 483. ELEVATION 237.63 | | 484. DEPTH 481 | |
| 485. ELEVATION 237.63 | | 486. DEPTH 237.63 | | 487. ELEVATION 237.63 | | 488. DEPTH 485 | |

Hole No. W-5

| DRILLING LOG | | DIVISION | INSTALLATION | SHEET 1 OF 1 SHEETS | |
|--|-------------|----------|--|--------------------------|--|
| 1. PROJECT Pearl River Cut-off | | | 10. SIZE AND TYPE OF BIT Hollow stem auger | | |
| 2. LOCATION (Coordinates or Station) 5 + 92 5 ft west of | | | 11. DATUM FOR ELEVATION SHOWN (TBM or MSL) MSL | | |
| 3. DRILLING AGENCY Waterways Experiment Station | | | 12. MANUFACTURER'S DESIGNATION OF DRILL Failing | | |
| 4. HOLE NO. (As shown on drawing title and file number) | | W-5 | 13. TOTAL NO. OF OVER-BURDEN SAMPLES TAKEN 0 | DISTURBED --- | UNDISTURBED --- |
| 5. NAME OF DRILLER Clyde Drake | | | 14. TOTAL NUMBER CORE BOXES 0 | | |
| 6. DIRECTION OF HOLE <input checked="" type="checkbox"/> VERTICAL <input type="checkbox"/> INCLINED --- DEG. FROM VERT. | | | 15. ELEVATION GROUND WATER 246.00 ft (16 Dec) | | |
| 7. THICKNESS OF OVERBURDEN 31.5 ft | | | 16. DATE HOLE STARTED 16 Dec 81 | COMPLETED 17 Dec 81 | |
| 8. DEPTH DRILLED INTO ROCK --- | | | 17. ELEVATION TOP OF HOLE 262.90 ft (MSL) | | |
| 9. TOTAL DEPTH OF HOLE 31.5 ft | | | 18. TOTAL CORE RECOVERY FOR BORING 0 % | | |
| | | | 19. SIGNATURE OF INSPECTOR Charlie Weller | | |
| ELEVATION ft, MSL | DEPTH ft | LEGEND | CLASSIFICATION OF MATERIALS (Description) | 1. CORE RECOVERY % | REMARKS (Drilling time, water loss, depth of weathering, etc., if significant) |
| 262.90 | b | c | garbage with a 1.0 ft clay soil cover | | Used hollow stem auger to drill the hole. Woman's stocking placed around well screen to keep fine grain materials out of PVC pipe. Silt allowed to collapse around the well screen as auger pulled out of hole. The hole was drilled under a power line, so the mast was not raised. |
| 252.50 | 10 | | silt-gray with some fine sand, changes to tan color below the water table (16.9 ft); pea gravel <1 ft thick at the base of the silt | | Water table at 16.9 ft. |
| 233.90 | 30 | | clay-green, fossiliferous, stiff, plastic (Yazoo clay) | | |
| 231.40 | | | Bottom of hole at 31.5 ft | | |

Hole No. W-6

| DRILLING LOG | | DIVISION | | INSTALLATION | | SHEET 1 OF 1 SHEETS | |
|---|-------------|--|--|---|-------------------------------|---|----------------|
| 1. PROJECT Pearl River Cut-off | | 2. LOCATION (Coordinates or Station) 3 + 32 on 6 | | 3. MANUFACTURER'S DESIGNATION OF DRILL Failing | | 10. SIZE AND TYPE OF BIT Hollow stem auger | |
| 4. DRILLING AGENCY Waterways Experiment Station | | 5. HOLE NO. (As shown on drilling info) and info number | | 6. TOTAL NO. OF OVER- BURDEN SAMPLES TAKEN | | 7. DISTURBED | 8. UNDISTURBED |
| W-6 | | | | | | 0 | 0 |
| 9. NAME OF DRILLER A. McNamara | | 10. TOTAL NUMBER CORE BOXES 0 | | 11. DAY AND ELEVATION SHOWN (TBM or MSL) MSL | | 12. ELEVATION GROUND WATER 244.53 ft (19 Dec) | |
| 11. DIRECTION OF HOLE <input checked="" type="checkbox"/> VERTICAL <input type="checkbox"/> INCLINED --- DEG. FROM VERT. | | 12. DATE HOLE 19 Dec 81 | | 13. STARTED 19 Dec 81 | | 14. COMPLETED 21 Dec 81 | |
| 13. THICKNESS OF OVERBURDEN 42.0 ft | | 15. ELEVATION TOP OF HOLE 273.53 ft (MSL) | | 16. TOTAL CORE RECOVERY FOR BORING 0 | | 17. SIGNATURE OF INSPECTOR Charlie White | |
| 18. DEPTH DRILLED INTO ROCK --- | | 19. REMARKS (Drilling time, water loss, depth of weathering, etc., if significant) | | | | | |
| 18. TOTAL DEPTH OF HOLE 42.0 ft | | | | | | | |
| ELEVATION ft, MSL | DEPTH ft | LEGEND | CLASSIFICATION OF MATERIALS (Description) | 5. CORE RECOVERY % | 6. CORE SAMPLE NO. 1 | REMARKS (Drilling time, water loss, depth of weathering, etc., if significant) | |
| 273.53 | 5 | | garbage with 1 ft of sandy silt soil cover | | | An open face auger was used to open the hole to the water table (29.0 ft) then 6" casing was installed and a sand bailer used to open the hole to 38.0 ft (top of Yazoo clay). The casing was pulled and hollow stem auger was used to drill into the clay. The sand was allowed to collapse around the well screen. | |
| 259.03 | 15 | | sand - gray, fine to medium silty sand; grades downward into a medium sand with some fine sand and silt, changes to white color below the water table (29.0 ft); pea gravel in lower 1 ft. | | | | |
| 235.53 | 30 | | clay - green, fossiliferous, stiff, plastic (Yazoo clay) | | | | |
| 231.53 | 40 | | Bottom of hole at 42.0 ft | | | | |

APPENDIX C

SAMPLE BORING LOGS DH-1 THROUGH DH-7

BORING LOG
FIELD DATA

Project Pearl River Cut-off Site Jackson, MS Date 17 Dec 81
 Location 0 + 36 on E Job No. DR-1
 Drill Rig Failing Inspector C. Whitten Operator A. McNamara Surface Elevation 274.48 ft Boring No. DR-1

| SAMPLE NUMBER | DATE TAKEN | STRATUM | DRIVE | SAMPLE | TYPE OF SAMPLER | BLows | CLASSIFICATION AND REMARKS |
|---------------|------------|---------|-------|--------|-----------------|--------------|---|
| | | FROM | TO | FROM | TO | | |
| 1 | 18 Dec | 18.0 | --- | 20.0 | 21.5 | 1-3/8" spoon | 3-3-3 clay, gray with iron stain and vegetation matter |
| 2 | --- | --- | --- | 25.0 | 26.5 | 4-4-4 | clay, gray soft |
| 3 | --- | --- | --- | 30.0 | 31.5 | 9-9-19 | sand-silty, fine with silt lenses |
| 4 | --- | 38.0 | 35.0 | 36.5 | | 25-4-3-49 | sand-fine to medium, white |
| 5 | 38.0 | --- | 40.0 | 41.5 | | 8-10-18 | Yazoo clay - green, fossil. stiff, plastic |
| 6 | --- | --- | 45.0 | 46.5 | | 5-11-22 | Yazoo clay |
| 7 | --- | --- | --- | 50.0 | 51.5 | 15-20-27 | Yazoo clay |
| 8 | --- | --- | 55.0 | 66.5 | | 10-14-18 | Yazoo clay |
| 9 | 18 Dec | 61.5 | 60.0 | 61.5 | 1-3/8" spoon | --- | Yazoo clay (splitspoon broke) |
| | | | | | | | NOTE: Pea gravel in top of sample #5 (cuttings) |

BORING LOG **FIELD DATA**

| | | | | | |
|-----------|---------------------|-----------|-------------|-------------|------------------------------------|
| Project | Pearl River Cut-off | Site | Jackson, MS | Date | 8 Jan 82 |
| Location: | 0 + 30 ft | W of | | Job No. | |
| Drill Rig | Failing | Inspector | C. Whitten | Operator C. | Drake |
| | | | | Surface E1 | 271.90 ft Boring No. DH-2 (MSL) |

WES FORM 819 EDITION OF NOV 1971 MAY BE USED
JAN 74

BORING LOG
FIELD DATA

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|-----------|-------------|----------------------|----------|-------------|------------|---------------------------|
| Project | Pearl River | Cut-off | Site | Jackson, MS | Date | 19 Jan 82 |
| Location | 7 + 58 | 114 ft W of E | | | | Job No. |
| Drill Rig | Failing | Inspector C. Whitten | Operator | C. Drake | Surface El | 276.15 ft Boring No. DH-4 |
| | | | | | (MSL) | |

BORING LOG

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| Project | Pearl River | Cut-off | Site | Jackson, MS | Date | 21 Jan 82 |
| Location | 10 + 35 12 ft | E of G | | | Job No. | |
| Drill Rig | Failing | Inspector C. Whitten | Operator | C. Drake | Surface El | 257.62 ft Boring No. DH-5 |

| SAMPLE NUMBER | DATE TAKEN | STRATUM | SAMPLE | | TYPE OF SAMPLER | BLOWS | CLASSIFICATION AND REMARKS |
|---------------|------------|---------|--------|-----|-----------------|-------|--|
| | | | FROM | TO | | | |
| 1 | 22 Jan | 0 | 4.0 | | 1-3/8" spoon | 50+ | Material pushed in by dozer hit root - no sample |
| 2 | | 4.0 | --- | 5.0 | 6.5 | | |
| 3 | | --- | 7.2 | 8.7 | | | |
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WES FORM 819 EDITION OF NOV 1971 MAY BE USED
10/74

BORING LOG
FIELD DATA

| Project | | Pearl River Cut-off | Site | | Jackson, MS | Date | | 23 Jan 82 |
|---------------|------------|----------------------|-----------|------------|-----------------|--------------|-----------------------------|--|
| Location | | 15 + 46 30 ft E of 2 | | | | Job No. | | |
| Drill Rig | | Failing | Inspector | C. Whitten | Operator | C. Drake | Surface El | 282.32 |
| | | | | | | | Boring No. | Di-6 |
| SAMPLE NUMBER | DATE TAKEN | STRATUM | DRIVE | SAMPLE | TYPE OF SAMPLER | BLOWS | CLASSIFICATION AND REMARKS | |
| | | FROM | TO | FROM | TO | | | |
| 1 | 23 Jan | 0 | 1.0 | | | | clay soil cover | |
| | | 1.0 | 33.4 | | | | landfill material (garbage) | |
| | | 33.4 | --- | 32.5 | 34.0 | 1-3/8" spoon | 4-7-10 | 0.2 ft garbage mixed with sand |
| | | | | | | | --- | 0.5 ft brown plastic clay |
| | | | | | | | --- | 0.1 ft fine to medium gray sand |
| | | | | | | | 8-5-2 | no sample in spoon-fine sand in catcher |
| 2 | 23 Jan | --- | 45.0 | 46.5 | | | No Blows | same as 40.0 to 41.5 |
| | | --- | --- | | | | 4-5-8 | sand - fine, gray silty - very |
| | | | | | | | --- | little sample in spoon |
| 3 | 25 Jan | --- | 50.0 | 51.5 | | | 6-9-4 | sand - fine, gray, silty |
| 4 | | --- | 58.0 | 55.0 | 56.5 | | 20-20-30 | sand-fine to medium, white, clean |
| 5 | | 58.0 | --- | 60.0 | 61.5 | | 17-20-34 | Yazoo clay-green, fossil, stiff, plastic |
| 6 | 25 Jan | --- | 66.5 | 65.0 | 66.5 | 1-3/8" spoon | 18-24-50+ | Yazoo clay-green, fossil. stiff, plastic |
| | | | | | | | | NOTE: Pea gravel in cuttings in top of sample #5 |

BORING LOG
FIELD DATA

Project Pearl River Cut-off Site Jackson, MS Date 25 Jan 82
 Location 14 + 18 148 ft E of Job No.
 Drill Rig Failling Inspector C. Whitten Operator C. Drake Surface El 286.83
 Boring No. DH-7

| SAMPLE NUMBER | DATE TAKEN | STRATUM FROM | DRIVE TO | SAMPLE FROM | SAMPLE TO | TYPE OF SAMPLER | BLOWS | CLASSIFICATION AND REMARKS |
|---------------|-------------|-----------------|-------------|----------------|--------------|--------------------|----------|-------------------------------------|
| 1 | 25 Jan 0 | 1.0 | | | | | | clay soil cover |
| 2 | 26 Jan 1.0 | 36.0 | | | | | | landfill material (garbage) |
| 3 | 26 Jan 36.0 | 35.0 | 36.5 | 36.0 | 36.5 | 1-3/8" spoon | 17-17-13 | sand-fine. gray. silty |
| 4 | — | — | 40.0 | 41.5 | | | 10-8-3 | sand-fine. gray. silty - no sample |
| 5 | — | — | — | — | | | — | In spoon; used handi-wrap on |
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